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Transportation Impact Study

PROPOSED RESIDENTIAL SUBDIVISION

7370 Centre Road UXBRIDGE, ONTARIO

April 2021

Project No: NT-20-058

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NextEng Consulting Group Inc.

April 1, 2021

Attention: Tina Fang

Bridgebrook Corp. 7681 Highway 27, Unit 16 Woodbridge, ON L4L 4M5

Re: Transportation Impact Study

Proposed Residential Subdivision

7370 Centre Road, Township of Uxbridge

Our Project No. NT-20-058

Nextrans Consulting Engineers (a Division of NextEng Consulting Group Inc.) is pleased to present the enclosed Transportation Impact Study for the above noted site in support of a proposed draft plan of subdivision for a residential development.

The subject site is located at 7370 Centre Road, north of Bolton Drive, between Centre Road and Concession Road 6, in the Township of Uxbridge. The subject site is currently vacant. The proposed development consists of 590 residential dwelling units (including 461 single-detached, 69 townhouse and 60 linked units). Two full moves intersections are proposed onto Concession 6 and one full moves intersection is proposed onto Centre Road which will be aligned with future Maplebrook Drive extension, to service the proposed development.

The transportation study concludes that the proposed development can adequately be accommodated by the existing transportation network, transit network as well as the Transportation Demand Management measures and incentives recommended in this report. The Study also recommended appropriate traffic calming and safety measures that are consistent with Durham's Vision Zero Strategic Road Safety Action Plan.

We trust the enclosed sufficiently addresses your needs. Should you have any questions, please do not hesitate to contact the undersigned.

Yours truly,

Nextrans Consulting Engineers

A Division of NextEng Consulting Group Inc.

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Report Submission Record

Identification	Date	Description of issued and/or revision		
Final Report	April 1, 2021	For Final Submission		

EXECUTIVE SUMMARY

Nextrans Consulting Engineers (A Division of NextEng Consulting Group Inc.) was retained by Bridgebrook Corp. (the 'Client') to undertake a Transportation Impact Study in support of a proposed draft plan of subdivision for a residential development. The subject site is located at 7370 Centre Road, north of Bolton Drive, between Centre Road and Concession Road 6, in the Township of Uxbridge.

This Transportation Impact Study has been prepared in accordance with the Region of Durham Traffic Impact Study Guidelines, as well as the requirements outlined in the proposed terms of reference submitted to the Township of Uxbridge and Durham Region.

Proposed Development

The subject site is current vacant. The proposed development consists of 590 residential dwelling units (including 461 single-detached, 69 townhouse and 60 semi/linked units). For the purposes of this assessment, the proposed 60 linked residential dwelling units will be treated at townhouse units, which is consistent with the definition of the Institute of Transportation Engineers (ITE) Trip Generation Manual 10th Edition.

Proposed Development Access

Two full moves intersections are proposed onto Concession 6, with the most southerly intersection is located approximately 225 m north of the Bolton Drive/Concession 6, with the northerly intersection is located approximately 150 m north of the southerly intersection.

A full moves intersection is also proposed onto Centre Road, which is located approximately 400 m north of the Bolton Drive/Centre Road intersection and to be aligned with the future Maplebrook Drive extension located on the east side of Centre Road.

The intersection capacity analysis indicates that all three proposed development intersections are expected to operate at acceptable levels of service with minimum delays or queues. The recommended intersection configurations and traffic controls include:

- 1. Concession 6/Site Intersection #1:
 - a. A shared left/right lane for westbound direction;
 - b. A shared through/right lane for the northbound and a shared through/left lane for the southbound direction; and
 - c. Stop controlled on the westbound direction (minor approach)
- 2. Concession 6/Site Intersection #2:
 - a. A shared left/through/right lane for eastbound and westbound directions:
 - b. A shared left/through/right lane for the northbound and southbound directions; and
 - c. Stop controlled on the westbound direction (minor approach)
- 3. Centre Road/Site Intersection #3/Maplebrook Drive Extension:
 - a. A shared left/through/right lane for eastbound and westbound directions;
 - b. A shared left/through/right lane for the northbound and southbound directions; and
 - c. Stop controlled on the Maplebrook Drive Extension and Site Intersection #3

Transportation Analysis

Based on the analysis noted above, the proposed development is expected to generate:

- 393 total two-way trips (97 inbound and 296 outbound) and 515 total two-way trips (325 inbound and 190 outbound) during the AM and PM peak hours, respectively;
- 361 two-way auto trips (89 inbound and 272 outbound) and 474 two-way auto trips (299 inbound and 175 outbound) during the AM and PM peak hours, respectively; and
- 32 two-way none single-occupant-vehicle trips (8 inbound and 24 outbound) and 41 two-way single-occupant-vehicle trips (26 inbound and 15 outbound) during the AM and PM peak hours, respectively.

Auto Mode Assessment

The intersection capacity analysis indicates that under existing, future background and future total conditions, all the intersections considered in the Study are expected to operate at acceptable levels of service. No physical improvements to the area road network and intersections are required for this horizon year.

Active Transportation Mode Assessment

Walking

As indicated, currently, there are some sidewalks located on one side of the street along Centre Road, Bolton Drive and Oakside Drive. Sidewalks are located on both sides of the street along Brock Street W east of Quaker Village Drive/S Balsam Street through the downtown area. Sidewalks are generally in good conditions.

Based on Nextrans' review and assessment of the proposed draft plan of subdivision for the proposed development, the following recommendations are provided:

- Sidewalks be provided at least on one side of all internal streets. If possible, sidewalks be provided on both sides of the main east-west streets, if appropriate;
- Sidewalk be provided on the west side of Centre Road from the northerly limit of the proposed development to connect with the existing sidewalk on Bolton Drive and Centre Road south of Bolton Drive;
- Sidewalk be provided on the east side of Concession 6 from the northerly limit of the proposed development to connect with the existing sidewalk on the south side of Bolton Drive; and
- Two sidewalk/cycling connections provided between Bolton Drive and proposed development to connect with the existing sidewalk on the south side of Bolton Drive and sidewalks on Quaker Village Drive.

Cycling

As indicated, under the existing conditions, there are no dedicated bicycle lanes along the main streets in the area. However, there are some existing trails such as Maple Bridge Trail, Quaker Trail, Butternut Trail and S Balsam Trail in the area.

It is Nextrans' understanding that the Township of Uxbridge initiated the Active Transportation Plan project (ATP) and has retained WSP Canada Group Limited with Share the Road Cycling Coalition to support the development of the Active Transportation Plan in the Township. The project is scheduled to completed by the end of 2020. Based on the information provided on the Township's website and notice of study commencement, the objectives of the Project are:

- Expand educational and promotional initiatives that raise awareness of active transportation opportunities for people of all ages and abilities;
- Develop a continuous Township-wide active transportation network that connects to all communities within Uxbridge and builds upon existing trail systems found within the Township; and

Collaborate with the Active Transportation Committee, the Trails Committee and the Accessibility Advisory
Committee as well as local municipal stakeholders, residents and Durham Region staff to ensure the community's
interests are addressed in the plan.

An external stakeholder workshop was held on Thursday November 5th, 2020 to get the public feedback on the project. Nextrans has reviewed and provided our own assessment and recommendations related to the proposed development based on the feedbacks and objectives of the ATP project

Transit Mode Assessment

Based on the information currently provided on the Township of Uxbridge website, Durham Region Transit offers three bus routes in the Township of Uxbridge: Route 601, Route 950 and Route 960. GO Transit also offers bus service and bus stops along Toronto Street in Uxbridge and Highway 47 in Goodwood.

However, based on the current information provided on Durham Region Transit website and system map, only bus route 905D is provided in the Township of Uxbridge at this time due to current COVID-19 pandemic situation.

The proposed development is expected to generate 32 two-way none single-occupant-vehicle trips (8 inbound and 24 outbound) and 41 two-way single-occupant-vehicle trips (26 inbound and 15 outbound) during the AM and PM peak hours, respectively.

The analysis indicates that no improvements are required to the existing active transportation and Durham Region Transit/GO Bus service in the area to accommodate the proposed development and this horizon year.

Durham's Vision Zero Strategic Road Safety Action Plan

Vision Zero

Vision Zero is a Swedish program that successfully halved traffic death using failsafe systems of road design, vehicle design and speed control. It aims to remove the possibility of any death or serious injury from transportation and road systems. No loss of life resulting from a motor vehicle collision is acceptable.

The context of Vision Zero in Canada, Durham Region and the Town of Uxbridge is described below:

- Canada https://visionzero.ca
 - Spreads awareness about Vision Zero and includes guides/toolkits for implementation
- Durham Region https://www.durham.ca/en/living-here/vision-zero.aspx
 - Durham has a Strategic Road Safety Action Plan that is fundamentally based on Vision Zero
 - o Goal: 10% reduction of fatal and injury collisions within five years (2019-2023).
 - Lists possible (but generalized) actions to meet this goal for intersections, pedestrians, cyclists, distracted driving, and more
 - Article from Nov. 2020 says that number of collisions are down but fatalities are unchanged
- In Uxbridge
 - o On Apr. 6, 2020, Durham Region staff presented Vision Zero and various stats to the Uxbridge General Purpose and Administration Committee. This was Agenda Item 10.1 and moved by Councillor Snooks
 - Concession Rd 6 and Davis Dr has the greatest collision frequency in Uxbridge
 - Presentation does not include any improvements for this intersection strangely
 - Region proposing many Regional projects to implement Red Light Cameras, Roundabouts, and more in Uxbridge.
 - Goodwood and Downtown Uxbridge are the main areas receiving these improvements

- The only nearby road improvement project is a roundabout at Reg. Hwy 47 (Toronto St) at Concession Rd 6 (construction in 2025, pending Region approval)
- o Uxbridge Council endorsed the meeting minutes from Apr. 6 (as Item 9.1) on Apr. 27. Moved by Councillor Popp and seconded by Councillor Snooks.

Vision Zero for the Proposed Development

As requested by the Durham Region staff, Nextrans will also recommend the proposed development to include the following requirements from Durham's Vision Zero Strategic Road Safety Action Plan. These requirements were based on Nextrans' review of Durham Region's Strategic Road Safety Action Plan (<u>Durham Region staff presented Vision Zero</u>). These requirements are based on the context of the proposed development.

Roundabout

 It is anticipated that the forecast traffic volumes at the proposed internal intersections and external intersections are very low, which will not meet the warrant for roundabout treatments. It is Nextrans' opinion that the stop signs and associated intersection treatment recommended in this Study would address these concerns

Traffic Signal Treatments

 As the traffic signals are not warranted for the proposed intersections as part of the proposed development, no traffic signal treatments are required at this time

Tighter Curb Radii

- o Provide smaller turning radii at all internal and external intersections to increase pedestrian safety by minimizing the crossing distances, increasing pedestrian visibility, and decreasing vehicle turning speed
- o The proposed locations with recommended tight turn radii are illustrated in Figure 19 of this Study

Roadway Design to Discourage Speeding

 Minimize the lane widths, edge lines, curb and gutter for all internal roads, as well as on Concession Road 6 and Centre Road along the frontage of the proposed development to discourage aggressive driving

Lane Widening for Truck

o It is recommended that the proposed development to maintain a minimum of 1.2 m to 1.5 m paved shoulder width along the frontage of the subject site along Concession Road 6 and Centre Road.

Flexible In-Road Warning Signs

o Given that Nextrans recommended to minimize the lane widths for all internal roads, installation of flexible in-road warning signs to guide traffic into a specific lane are not required. It is recommended that the Region and the Town consider install these flexible in-road warning signs for Concession Road 6, Centre Road, Bolton Drive, Brock Street W and Quaker Village Drive in the area

Transverse Rumble Strips

 It is Nextrans' opinion that this is not required for the proposed development. It is recommended that the Town consider this treatment for Centre Road southbound and northbound north and south of Bolton Drive/North Street

Zebra Crosswalks

It is recommended that raised and enhanced crosswalk markings (ladder/zebra) at all all-way stop intersections and approaches with stop signs, as well as at all pedestrian crossing locations

Crossrides

o Given that there is no update yet on the WSP's Active Transportation Plan at the time of preparation of this Transportation Impact Study, Nextrans recommends that these requirements be reviewed as part of the

detailed and engineering design at that time. It is recommended that the proposed development protect for this treatment in the future.

Paved Shoulders

 It is recommended that the proposed development to maintain a minimum of 1.2 m to 1.5 m paved shoulder width along the frontage of the subject site along Concession Road 6 and Centre Road.

It should be noted that since this Transportation Impact Study is prepared to support the proposed Official Plan and Zoning By-law Amendment applications, this Study is not a detailed designed or engineering study. Therefore, most of these requirements will be addressed through the detailed site design and engineering submission at the subsequent stage of the proposed development.

Transportation Demand Management Measures and Incentives

The TDM measures and incentives related to the proposed development have been assessed and recommended in Section 9 of this report to support active transportation and transit, to meet the objectives and requirements of the City's and the Region's Guidelines.

Servicing Truck Turning Movement

AutoTURN software was used to generate the turning movement templates for servicing trucks (TAC-HSU). More detailed vehicle turning movements will be provided as part of the future application, if required.

Study Conclusions and Recommendations

Based on the Study findings and conclusions, the following recommendations are provided:

- The proposed development implements the TDM measures and incentives identified in this report to support
 active transportation and transit and to reduce the numbers of single-occupant-vehicle trips to and from the
 proposed development
- The proposed development implements Durham's Vision Zero Strategic Road Safety Action Plan requirements for the internal roads and intersections, as well as the external intersections onto Concession Road 6 and Centre Road, as recommended in Sections 7.6 and 7.7 of this Study
- The proposed development provides the recommended internal and external active transportation network
- The proposed development provides the recommended traffic control types for the internal and external intersections
- No additional physical improvements for the area road network and intersection to accommodate the proposed development under the future total conditions for the horizon year considered

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1.0 INTRODUCTION

Nextrans Consulting Engineers (A Division of NextEng Consulting Group Inc.) was retained by Bridgebrook Corp. (the 'Client') to undertake a Transportation Impact Study in support of a proposed draft plan of subdivision for a residential development. The subject site is located at 7370 Centre Road, north of Bolton Drive, between Centre Road and Concession Road 6, in the Town of Uxbridge.

This Transportation Impact Study has been prepared in accordance with the Region of Durham Traffic Impact Study Guidelines, as well as the requirements outlined in the proposed terms of reference submitted to the Township of Uxbridge and Durham Region. The relevant correspondence is included in **Appendix A**.

The location of the proposed development is illustrated in **Figure 1**.

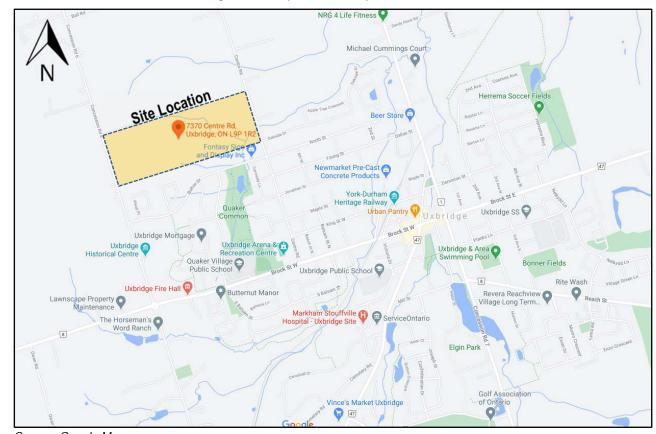


Figure 1 – Proposed Development Location

Source: Google Map

The subject site is current vacant. The proposed development consists of 590 residential dwelling units (including 461 single-detached, 69 townhouse and 60 semi/linked units). For the purposes of this assessment, the proposed 60 linked residential dwelling unit will be treated at townhouse units, which is consistent with the definition of the Institute of Transportation Engineers (ITE) Trip Generation Manual 10th Edition.

Two full moves intersections are proposed onto Concession 6, with the most southerly intersection is located approximately 225 m north of the Bolton Drive/Concession 6, with the northerly intersection is located approximately 150 m north of the southerly intersection.

A full moves intersection is also proposed onto Centre Road, which is located approximately 400 m north of the Bolton Drive/Centre Road intersection and will be aligned with the future Maplebrook Drive extension on the east side of Centre Road.



Figure 2 illustrates the proposed development conceptual plan.

EXISTING AGRICULTURAL

NHS
1.54ha
1.54ha
1.70ha
1.70ha
1.70ha
1.55ha
1.5

Figure 2 – Conceptual Plan

2.0 EXISTING TRAFFIC CONDITIONS

2.1. Existing Road Network

The existing road network, lane configuration and existing traffic control for the study area are shown in **Figure 3** (Existing Lane Configurations). The details area described below:

- Centre Road: is a north-south collector road as identified in Schedule 'A' of the Township of Uxbridge Official Plan (Consolidated January 2014). Centre Road generally has a two-lane urban cross-section south of Oakside Drive with sidewalk on one side of the road, and rural cross-section north of Oakside Drive with no sidewalk. It generally maintains a posted speed limit of 50 km/h.
- Bolton Drive/North Street: is an east-west local road as identified in Schedule 'A' of the Township of
 Uxbridge Official Plan (Consolidated January 2014). It generally has two-lane urban cross-section west of
 North Street with sidewalks on both sides of the street, and rural cross-section east of North Street with no
 sidewalk. It generally maintains a posted speed limit of 50 km/h.
- Quaker Village Drive: is a north-south collector road as identified in Schedule 'A' of the Township of Uxbridge Official Plan (Consolidated January 2014). It generally has two-lane urban cross-section with sidewalks on both sides of the street. It maintains a posted speed limit of 40 km/h.
- Oakside Drive: is an east-west local road as identified in Schedule 'A' of the Township of Uxbridge Official Plan (Consolidated January 2014). It has two-lane urban cross-section with sidewalk located on the north side of the street. It generally maintains a posted speed limit of 50 km/h.
- Concession Road 6: is a north-south collector road as identified in Schedule 'A' of the Township of Uxbridge Official Plan (Consolidated January 2014). It generally has two-lane rural cross-section with no sidewalk. It maintains a posted speed limit of 60 km/h near the subject site.
- Brock Street W/Regional Road 8: is an east-west arterial road as identified in Schedule 'A' Map 'A2' of the Regional of Durham Official Plan (Consolidated May 2020). It generally has two lane rural cross-section with no sidewalk west of Concession Road 6, and has a two-lane urban cross-section east of Concession Road 6 with sidewalk located on the south side east of Norm Goodspeed Drive and both sides east of Quaker Village



Drive/S Balsam Street. It generally maintains a posted speed limit of 80 km/h west of Concession 6 and 50 km/h east of Concession Road 6.

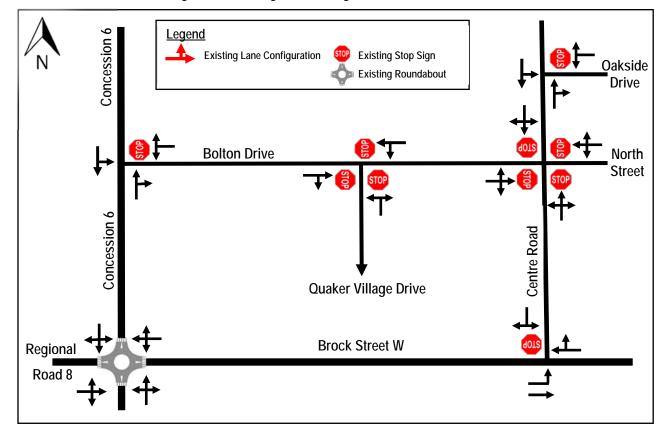


Figure 3 – Existing Lane Configuration and Traffic Control

2.2. Existing Active Transportation Assessment

Walking

Currently, there are some sidewalks located on one side of the street along Centre Road, Bolton Drive and Oakside Drive. Sidewalks are located on both sides of the street along Brock Street W east of Quaker Village Drive/S Balsam Street through the downtown area. Sidewalks are generally in good conditions. **Figure 4** illustrates the existing active transportation network in the study area.

Cycling

Under the existing conditions, there are no dedicated bicycle lanes along the main streets in the area. However, there are some existing trails such as Maple Bridge Trail, Quaker Trail, Butternut Trail and S Balsam Trail in the area.

It is Nextrans' opinion both the sidewalk and cycling network can be improved in the future as part of the Town wide capital project to connect the community better.

As part of this Study, Nextrans will provide assessment and recommendations for the proposed development to provide appropriate internal and external sidewalk and cycling network to accommodate the proposed development and to connect with the existing communities to encourage future residents to take active modes of transportation.





Figure 4 – Existing Active Transportation Network in the Study Area

Source: Google Map

2.3. Existing Transit Assessment

Based on the information currently provided on the Township of Uxbridge website, Durham Region Transit offers three bus routes in the Township of Uxbridge: Route 601, Route 950 and Route 960. GO Transit also offers bus service and bus stops along Toronto Street in Uxbridge and Highway 47 in Goodwood.

However, based on the current information provided on Durham Region Transit website and system map, only bus route 905D is provided in the Township of Uxbridge at this time due to current COVID-19 pandemic situation.

Below is a general description of the Durham Region Transit and GO Transit bus routes. It is anticipated that once the pandemic is under control or over, some of these routes may return to normal operation in the Township of Uxbridge.

- Bus Route 950 This bus route operates Uxbridge, Port Perry and Ontario Tech/Durham College North Campus.
- Bus Route 601 This bus route operates Uxbridge, Leaskdale, Sunderland, Manilla, Cannington and Beaverton.
- **Bus Route 960** This bus route operates between Uxbridge and Davis Drive/Hwy 404 GO Bus and York Region Transit/Viva Terminal.
- Bus Route 71 GO Transit This bus route operates between Uxbridge and Goodwood.

2.4. Existing Traffic Volumes

Existing traffic volumes at the study area intersections were undertaken by Spectrum on Wednesday October 28, 2020 during the morning (7:00 a.m. to 10:00 a.m.) and afternoon (4:00 p.m. to 7:00 p.m.) peak periods. As requested by Durham Region staff, the Regional Road 8/Brock Street/Concession 6 intersection (existing roundabout) was also



counted and will be reflected in the analysis. As these counts are conduced in 2020, it will be adjusted and normalized using the previous count factors or growth rate to account for the pandemic situation, where appropriate.

Nextrans also obtained existing traffic turning movement counts for the Regional Road 8/Brock Street/Concession 6 intersection (Thursday May 3, 2018) and the Brock Street W/Centre Road intersection (Wednesday November 13, 2013). The 2018 counts can be used as it is less than 2 years old, however, the 2013 counts will only be used for comparison and checking purposes.

Turning movement counts are summarized in **Appendix B**. The existing volumes are illustrated in **Figure 5**.

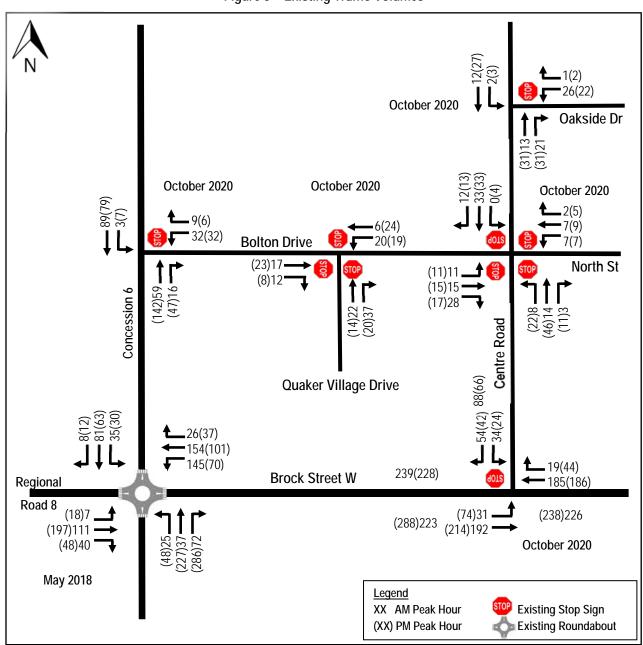


Figure 5 – Existing Traffic Volumes

2.5. COVID-19 Pandemic Traffic Count Analysis and Adjustment

In light of the COVID-19 pandemic, there are some speculations and expectations that the traffic counts undertaking during the pandemic will be much lower than the pre-pandemic. This may be the case given that many offices and



employers allowed staff working from home. There is also a number of students learning online instead of attending classes. For these reasons, it is expected that there are less cars on the road during the peak period.

In order to understand the magnitude of the impact, Nextrans has conducted a review and comparison between the prepandemic counts and recent counts in October 2020. Given that Nextrans has obtained the previous counts dated May 2018 and November 2013 for the Regional Road 8/Brock Street/Concession 6 and Brock Street W/Centre Street, respectively, a 2% growth per annum (compounded) was applied to all movements at these two intersections. Based on this review, it is indicated that the north-south volumes along Concession 6 north of Regional Road 8/Brock Street are relatively comparable, with the exception of the northbound pm peak period, which sees a drop of about 100 vehicles or about 35%. In addition, the comparison indicates that there is a significant drop in traffic volume up to 50% at the Brock Street W/Centre Road intersection during the pandemic. Figure 6 illustrates the link volumes and intersection volumes comparison.

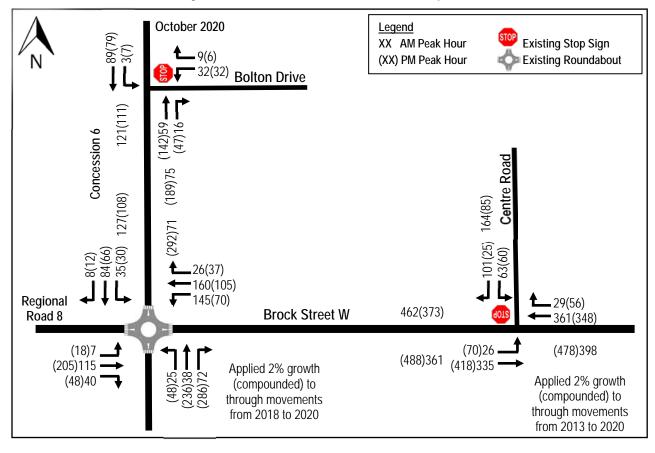


Figure 6 – Link and Intersection Volume Comparison

Based on the findings indicated above, the following approaches and adjustments will be applied to the existing traffic volumes to account for the pandemic:

- A 2% growth per annum (compounded) will be applied to the pre-pandemic traffic counts for all movements at the Regional Road 8/Brock Street/Concession 6 and Brock Street W/Centre Street;
- The turning movement volumes at the Concession 6/Bolton Drive intersection will be balance using the link volumes at the Regional Road 8/Brock Street/Concession 6 intersection
- 50% adjustment factor will be applied to all other intersections and for all movements.

It is Nextrans' opinion that the approaches and methodologies noted above are reasonable given the COVID-19 pandemic is still on-going in Uxbridge, Durham Region and Ontario. Adjusted turning movement counts are illustrated in **Figure 7**.



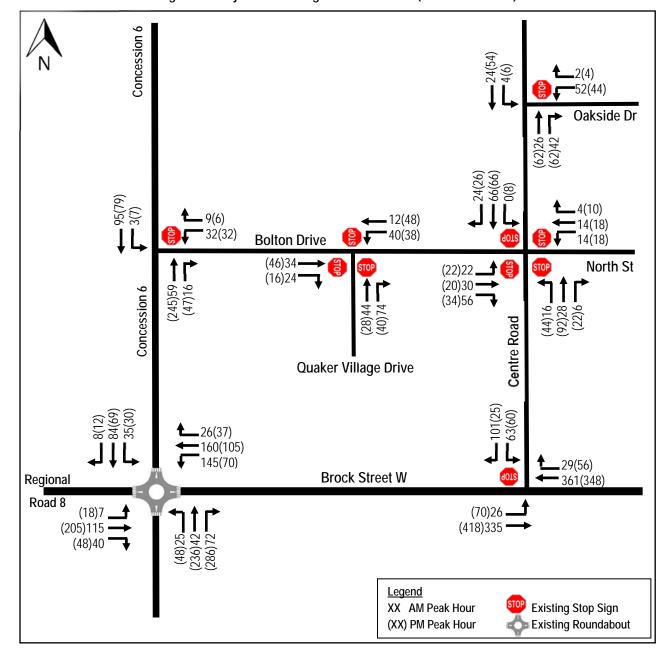


Figure 7 – Adjusted Existing Traffic Volumes (2020 Conditions)

2.6. Existing Traffic Assessment

The existing volumes in **Figure 7** were analyzed using Synchro Version 10 software. The methodology of the software and the result outputs are based on the procedures described and outlined in the Highway Capacity Manual (HCM), published by the Transportation Research Board.

The detailed results are provided in **Appendix C** and summarized in **Table 2**.



Table 1 – Existing Levels of Service

		Weekday AM Peak Hour			Week	Available		
Intersection	Movement	LOS (v/c)	Delay (s)	95 th Queue (m)	LOS (v/c)	Delay (s)	95 th Queue (m)	Storage (m)
Regional Road 8/	EB – LTR	A (0.24)	7	1	A (0.31)	7	1	300+
Brock Street W/	WB – LTR	A (0.36)	7	2	A (0.28)	8	1	250+
Concession 6	NB – LTR	A (0.17)	6	1	C (0.71)	18	6	300+
(Roundabout)	SB – LTR	A (0.19)	7	1	A (0.14)	6	0	580
Brock Street W/	EB – L	A (0.03)	8	1	A (0.08)	9	2	95
Centre Road	EB – T	A (0.23)	0	0	A (0.28)	0	0	95
(Unsignalized)	WB – TR	A (0.27)	0	0	A (0.27)	0	0	95
(Unsignalizeu)	SB – TR	C (0.46)	20	19	D (0.35)	25	12	185
Concession 6/	WB – LR	A (0.06)	10	1	B (0.06)	11	2	75
Bolton Drive	NB – TR	A (0.05)	0	0	A (0.18)	0	0	580
(Unsignalized)	SB – TL	A (0.00)	0	0	A (0.01)	1	0	300+
Bolton Drive/	EB – LTR	A (0.22)	9		A (0.10)	8		100
Centre Road/	WB – LTR	A (0.07)	8		A (0.07)	8		200
North Street	NB – LTR	A (0.11)	8		A (0.21)	9		135
(Unsignalized)	SB – LTR	A (0.19)	9		A (0.13)	8		105
Bolton Drive/	EB – TR	A (0.09)	8		A (0.08)	7		150
Quaker Village Dr	WB – TL	A (0.09)	8		A (0.12)	8		240
(Unsignalized)	NB – LR	A (0.18)	8		A (0.09)	7		300
Centre Road/	WB – LR	A (0.08)	10	2	A (0.06)	10	2	300+
Oakside Drive	NB – TR	A (0.05)	0	0	A (0.08)	0	0	105
(Unsignalized)	SB – TL	A (0.00)	1	0	A (0.00)	1	0	300+

Based on the intersection capacity analysis, under the existing traffic conditions, all the intersections considered are currently operating at acceptable levels of service. The expected queues can be accommodated with the current available storage, all v/c ratios are under 1.0 and the highest approach delays are less only 25 seconds. Therefore, no improvements are required at this time.

3.0 TRANSPORTATION PLANNING CONTEXT IN THE AREA

3.1. Land Use Context

As indicated, the subject site is currently vacant. The subject site is located north of the existing urban area north of Bolton Drive, between Centre Road and Concession 6. Based on Schedule "A" Land Use and Transportation Plan Uxbridge Urban Area (Uxbridge 2014 Official Plan), these lands are designated as future residential area.

Currently, the majority of the existing residential developments are located to the south and east of subject site, with some new and future developments are located to the east of Centre Road and north of North Street/Bolton Drive. The majority of the existing commercial, retails, employment and personal services are located in the downtown area at Brock Street W/Toronto Street/Main Street.

As the proposed development is consisting of 590 residential dwelling units (including 461 single-detached, 69 townhouses and 60 linked units), it is generally consistent with the existing developments and contexts of the community.

3.2. Transportation Planning Context

As indicated in Section 2, a complete network of sidewalk is generally provided along Bolton Drive, Centre Road, Brock Street W, Quaker Village Drive and Oakside Drive. Although sidewalks are mostly available on one side of the street, it is sufficient for the existing context. Currently, there is no existing on-street cycling lanes on the public street, however, there are existing trails in the area that connect to major destinations in the Township of Uxbridge. Under the current COVID-19 pandemic, there is only one Durham Region Transit Bus Route 905 servicing the area at this time. It is anticipated that more transit routes will be running again once the pandemic is over.



3.3. Township of Uxbridge Active Transportation Plan

The Township of Uxbridge initiated the Active Transportation Plan project (ATP) and has retained WSP Canada Group Limited with Share the Road Cycling Coalition to support the development of the Active Transportation Plan in the Township. The project is scheduled to completed by the end of 2020. Based on the information provided on the Township's website and notice of study commencement, the objectives of the Project are:

- Expand educational and promotional initiatives that raise awareness of active transportation opportunities for people of all ages and abilities;
- Develop a continuous Township-wide active transportation network that connects to all communities within Uxbridge and builds upon existing trail systems found within the Township; and
- Collaborate with the Active Transportation Committee, the Trails Committee and the Accessibility Advisory Committee as well as local municipal stakeholders, residents and Durham Region staff to ensure the community's interests are addressed in the plan.

An external stakeholder workshop was held on Thursday November 5th, 2020 to get the public feedback on the project. Nextrans has reviewed and provided our own assessment and recommendations related to the proposed development based on the feedbacks and objectives of the ATP project.

4.0 FUTURE BACKGROUND CONDITIONS

4.1. Analysis Horizon

It is anticipated that the proposed development will be completed by 2023. For the purposes of this assessment, a five-year horizon (2028) has been carried out for the study analysis. This is consistent with Durham Region Traffic Impact Study Guidelines.

4.2. Future Background Corridor Growth

Nextrans has reviewed various background transportation studies prepared for proposed background development applications in the area. In generally, a 2% growth per annum (compounded) or a total of 16% growth (from 2020 to 2028) is applied to the existing traffic volumes for both the Regional and Township roads.

Nextrans also reviewed the census profiles for the Township of Uxbridge. This information indicates that the population growth for the Township of Uxbridge between 2011 and 2016 is approximately 1.1%, or 0.22% per annum. Therefore, a 2% growth rate per annum (compounded) or a total of 16% growth is very conservative and will account for other developments in the Township of Uxbridge.

Nextrans has contacted and provided a study terms of reference to both the Region of Durham and the Township of Uxbridge. Nextrans has indicated that a 2% growth per annum (compounded) will be utilized for the study area intersections. **Figure 8** illustrates the 2025 background growth rate for the intersections considered.

4.3. Background Development Applications

A full review of active background developments within the study area was conducted based on the information extracted from the Township of Uxbridge website (https://www.uxbridge.ca/en/business-and-development/current-planning-applications.aspx).

This review indicates that none of the active development applications are located within the study area and the intersections considered in the analysis. Therefore, it is Nextrans' opinion that a 2% growth rate per annum (compounded) or a total of 16% growth will account for all of these developments in the Township of Uxbridge.



In order to estimate the future turning movements at the Centre Road/Maplebrook Drive extension intersection, site trip generation and distribution for the Mason Home, Maple Bridge Residential Subdivision Phase II will be estimated using the *Trip Generation Manual*, 10th Edition published by the Institute of Transportation Engineers (ITE), 2016 Transportation Tomorrow Survey (TTS) data and existing travel patterns at the existing intersections.

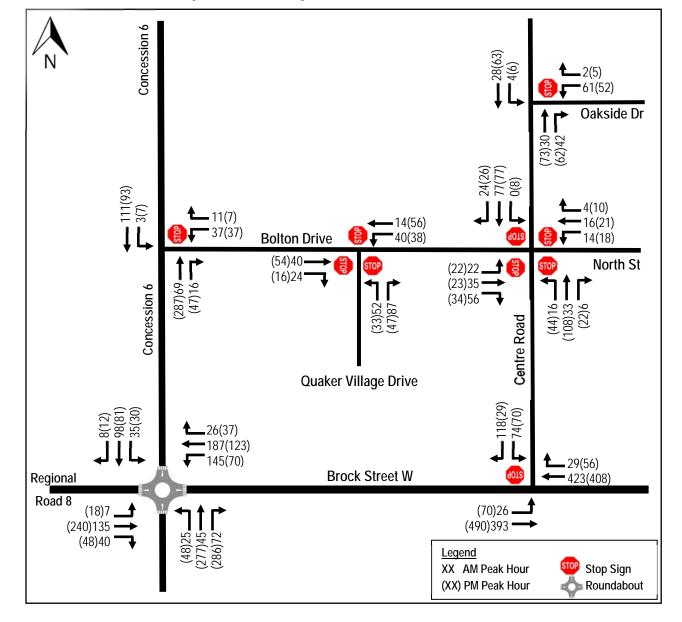


Figure 8 – 2028 Background Corridor Growth for the Area

Although the proposed Mason Homes proposed residential subdivision Phase II consists of 152 residential dwelling units, which also include single-family detached and semi-detached units, for the purposes of this assessment, it has been assumed that all 152 units are single-family detached. This assumption is conservative given that the single-family detached units typically generate more trips than the semi-detached units.

Based on our review of the *Trip Generation Manual, 10th Edition* published by the Institute of Transportation Engineers (ITE), the Land Use Code (LUC) 210 "Single-Family Detached Housing General Urban/Suburban" fitted curve equations and average have been utilized for the proposed development. The site trip generation is summarized in **Table 5**.

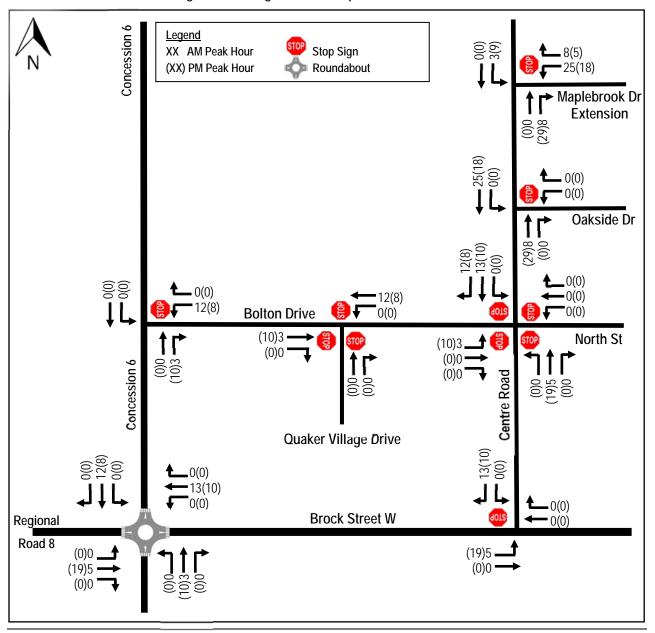


Table 2 – Site Trip Generation

ITE Land Use	Magnitude	Parameters	Morn	ing Peak	Hour	Afternoon Peak Hour		
TTE Latiu USe	(units)	Parameters	In	Out	Total	In	Out	Total
Single-Family Detached Housing LUC 210 General	152	Trip Rates AM - T = 0.71(X) + 4.80 PM - Ln(T) = 0.96Ln(X) + 0.20		0.55	0.74	0.63	0.37	1.00
Urban/Suburban		Total Trips	28	85	113	96	56	152
Modal Split (8%)			2	7	9	8	4	12
New Auto Trips			26	78	104	88	52	140

Based on the analysis noted above, the proposed development is expected to generate 104 total two-way trips (26 inbound and 78 outbound) and 140 total two-way trips (88 inbound and 52 outbound) during the AM and PM peak hours, respectively. **Figure 9** illustrating the Mason Home site generated traffic volumes. It should be noted that some of Mason Homes site traffic will go through the study area, other trips will use the other side of Maplebrook Drive extension, which will not be passing by the study area.

Figure 9 – Background Development Traffic Volumes





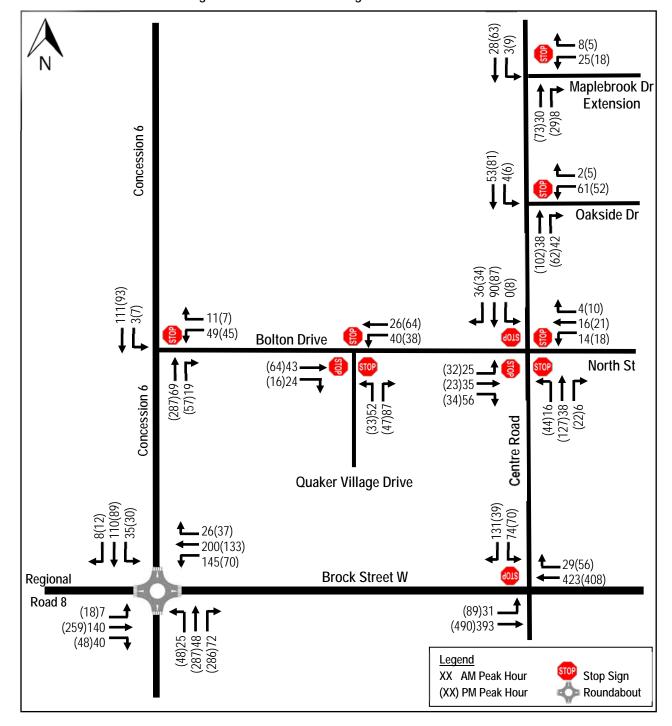


Figure 10 – 2028 Future Background Traffic Volumes

4.4. Future Background Traffic Assessment

The estimated 2028 future background traffic volumes are illustrated in **Figure 8**, and were analyzed using Synchro Version 10 software. The detailed calculations are provided in **Appendix D** and summarized in **Table 5**.



Table 3 – 2028 Future Background Levels of Service

		Weekday AM Peak Hour			Week	day PM Peal	(Hour	Available
Intersection	Movement	LOS (v/c)	Delay (s)	95 th Queue (m)	LOS (v/c)	Delay (s)	95 th Queue (m)	Storage (m)
Regional Road 8/	EB – LTR	A (0.28)	8	1	A (0.38)	8	2	300+
Brock Street W/	WB – LTR	A (0.41)	8	2	A (0.34)	9	1	250+
Concession 6	NB – LTR	A (0.18)	6	1	D (0.82)	26	9	300+
(Roundabout)	SB – LTR	A (0.24)	8	1	A (0.17)	6	1	580
Drack Chact W/	EB – L	A (0.04)	9	1	A (0.09)	9	2	95
Brock Street W/	EB – T	A (0.28)	0	0	A (0.31)	0	0	95
Centre Road	WB – TR	A (0.32)	0	0	A (0.29)	0	0	95
(Unsignalized)	SB – TR	C (0.66)	32	37	D (0.47)	31	19	185
Concession 6/	WB – LR	A (0.08)	10	2	B (0.10)	12	3	75
Bolton Drive	NB – TR	A (0.06)	0	0	A (0.22)	0	0	580
(Unsignalized)	SB – TL	A (0.00)	0	0	A (0.01)	1	0	300+
Bolton Drive/	EB – LTR	A (0.25)	9		A (0.13)	8		100
Centre Road/	WB – LTR	A (0.08)	9		A (0.07)	8		200
North Street	NB – LTR	A (0.13)	9		A (0.26)	9		135
(Unsignalized)	SB – LTR	A (0.28)	9		A (0.18)	8		105
Bolton Drive/	EB – TR	A (0.11)	8		A (0.11)	8		150
Quaker Village Dr	WB – TL	A (0.12)	8		A (0.14)	8		240
(Unsignalized)	NB – LR	A (0.22)	8		A (0.11)	8		300
Centre Road/	WB – LR	A (0.10)	10	3	A (0.06)	10	2	300+
Oakside Drive	NB – TR	A (0.06)	0	0	A (0.10)	0	0	105
(Unsignalized)	SB – TL	A (0.00)	1	0	A (0.00)	1	0	300+
Centre Road/	WB – LR	A (0.05)	9	1	A (0.03)	10	1	300+
Maplebrook Dr	NB – TR	A (0.03)	0	0	A (0.06)	0	0	280
(Unsignalized)	SB – TL	A (0.00)	1	0	A (0.01)	1	0	300+

Based on the intersection capacity analysis, under the 2028 future background traffic conditions, all the intersections considered are expected to operate at acceptable levels of service. Forecasted queues can be accommodated with the current available storage, v/c ratios are under 1.0 and the highest approach delays are expected to be approximately 32 seconds. Therefore, no improvements are required under this horizon year.

5.0 SITE TRAFFIC

5.1. Proposed Development

As indicated, the proposed development consists of 590 residential dwelling units (including 461 single-detached, 69 townhouse and 60 semi/linked units). The 2016 Transportation Tomorrow Survey (TTS), the catchment area and *Trip Generation Manual*, 10th Edition published by the Institute of Transportation Engineers (ITE) were reviewed to estimate the modal split, trip distribution and trip generation for the proposed development.

5.2. Modes of Travel Assessment in the Area

Since the subject site is currently vacant, the surrounding traffic zones have been utilized to analyze the existing modal split. For the purposes of this assessment, the inclusion of Traffic Zones 1313, 1316, 1317, 1318 is appropriate and reasonable. **Table 4** summarizes the travel modal split information. The detailed 2016 TTS data analysis is included in **Appendix E**.

Table 4 - Modes of Travel based on 2016 TTS Data

	Trips Made to the Township of Uxbridge								
Time	Auto Driver (including motorcycle)	Auto Passenger (including paid rideshare and taxi)	Transit/ GO	Cycle	Walk				
AM Peak Period (6:00 – 9:00)	69%	11%	4%	0%	16%				
PM Peak Period (4:00 – 7:00)	77%	15%	3%	1%	4%				



Based on the information provided above, currently the majority mode of transportation in the area is private automobile, which accounts for 69% during the morning and 77% during the afternoon peak periods. The none single-occupant-vehicle modal split (transit, walking and cycling) is 20% during the morning and only 8% during the afternoon peak period. It is Nextrans' opinion that, although the proposed development is not located as close to the downtown area as some of the traffic zones considered in the analysis above and therefore the walking mode will not be as high as 16%, however, it is anticipated that other none single-occupant-vehicle modes such as carpooling, paid rideshare (i.e. uber and taxi) and transit mode will be close to the percentage indicated above.

For the purposes of this assessment and to be conservative, an 8% modal split for all none single-occupant-vehicle modes such as walking, cycling, transit and carpooling/shared-ride will be applied to the proposed development and background developments in the area. Given that Nextrans will provide a comprehensive assessment and recommendations for the active transportation network within the proposed development, it is Nextrans' opinion that the proposed 8% modal split for all none single-occupant-vehicle mode can be achieved and justified.

5.3. Site Trip Generation

Based on Nextrans' review of the *Trip Generation Manual, 10th Edition* published by the Institute of Transportation Engineers (ITE), the Land Use Codes (LUC) 210 "Single-Family Detached Housing General Urban/Suburban" and (LUC) 220 "Multifamily Housing Low-Rise General Urban/Suburban" fitted curve equations and average have been utilized for the proposed development. The site trip generation is summarized in **Table 5**.

ITE Land Use	Magnitude Parameters -		Morning Peak Hour			Afternoon Peak Hour		
TTE Latiu USE	(units)	rai ai iletei S	In	Out	Total	In	Out	Total
Multifamily Housing (Low-Rise) LUC 220 General	129	Trip Rates AM - Ln(T) = 0.95Ln(X) - 0.51 PM - Ln(T) = 0.89Ln(X) - 0.02	0.11	0.36	0.47	0.36	0.21	0.57
Urban/Suburban		Total Trips	14	47	61	47	27	74
Single-Family Detached Housing LUC 210 General	461	Trip Rates AM - T = 0.71(X) + 4.80 PM - Ln(T) = 0.96Ln(X) + 0.20	0.18	0.54	0.72	0.6	0.36	0.96
Urban/Suburban		Total Trips	83	249	332	278	163	441
	•	•						
	Total Trips			296	393	325	190	515
None Singl	None Single-Occupant-Vehicle Modal Split (8%)			24	32	26	15	41
	New Auto Trips				361	299	175	474

Table 5 – Site Trip Generation

Based on the analysis noted above, the proposed development is expected to generate:

- 393 total two-way trips (97 inbound and 296 outbound) and 515 total two-way trips (325 inbound and 190 outbound) during the AM and PM peak hours, respectively;
- 361 two-way auto trips (89 inbound and 272 outbound) and 474 two-way auto trips (299 inbound and 175 outbound) during the AM and PM peak hours, respectively; and
- 32 two-way none single-occupant-vehicle trips (8 inbound and 24 outbound) and 41 two-way single-occupant-vehicle trips (26 inbound and 15 outbound) during the AM and PM peak hours, respectively.

5.4. Site Trip Distribution and Assignment

The 2016 Transportation Tomorrow Survey (TTS) data was reviewed for Traffic Zones 1313, 1316, 1317, 1318 in order to estimate the general trip distribution for the proposed development. **Table 6** summarizes general distribution based on the 2016 TTS data for the proposed development, with **Table 7** summarizing the site traffic assignment. It should be noted that the auto site trip distribution and assignment have been taken into consideration the 2016 TTS information and existing intersection turning movements, where appropriate.



Table 6 – Trip Distribution for Auto Mode

Toronto	Uxbridge	Durham Region	York Region	Peel Region	Others
10%	38%	17%	31%	1%	3%

Table 7 – Site Trip Distribution

General Direction (To/From)	Inbound	Outbound
North (via Concession 6, Centre Road and Concession 7)	5%	5%
South (via Concession 6, Centre Road and Concession 7/Main Street S)	25%	25%
East (Brock Street W/Regional Road 8, Davis Drive)	25%	25%
West (Brock Street W/Regional Road 8, Davis Drive and Toronto Street)	45%	45%
Total	100%	100%

Figure 11 illustrates the proposed development generated traffic volumes

Figure 11 – Site Traffic Volumes

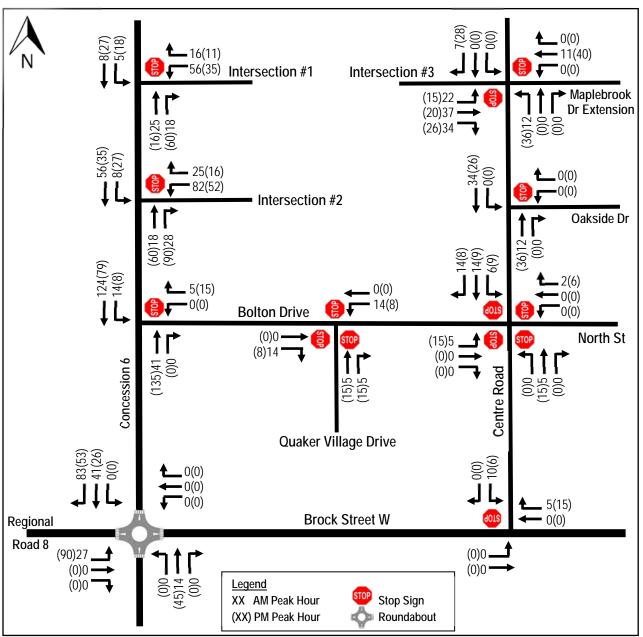




Figure 12 illustrating the 2028 future total traffic volumes (future background traffic volumes plus site generated traffic volumes).

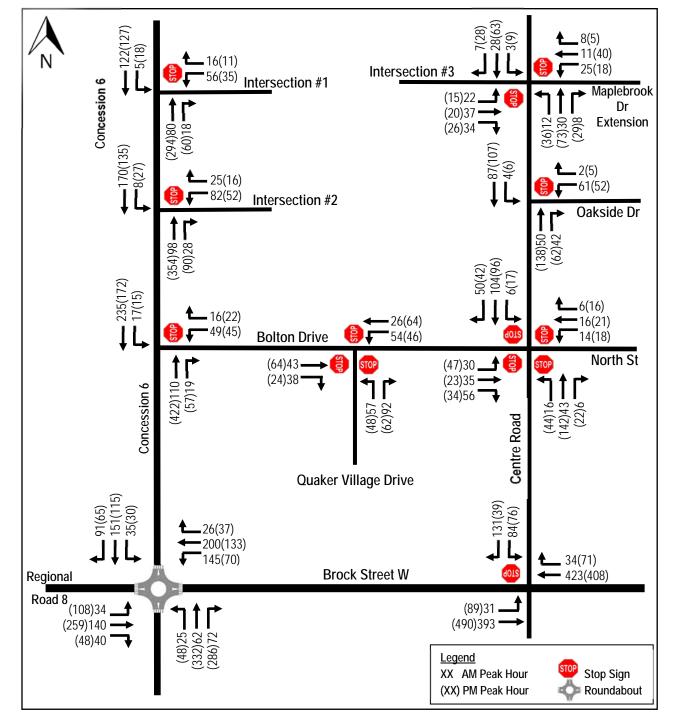


Figure 12 – 2028 Future Total Traffic Volumes

6.0 FUTURE TOTAL TRAFFIC CONDITIONS

6.1. Future Total Traffic Assessment for Auto Mode

The estimated 2028 future total traffic volumes (future background traffic volumes plus site generated traffic volumes) are illustrated in **Figure 12**, and were analyzed using Synchro Version 10 software. The detailed calculations are provided in



Appendix F and summarized in Table 11.

Table 8 - 2028 Future Total Levels of Service

		Weekday AM Peak Hour		Week	Available			
Intersection	Movement	LOS (v/c)	Delay (s)	95 th Queue (m)	LOS (v/c)	Delay (s)	95 th Queue (m)	Storage (m)
Regional Road 8/	EB – LTR	A (0.34)	9	2	B (0.50)	11	3	300+
Brock Street W/	WB – LTR	A (0.43)	9	2	B (0.39)	11	2	250+
Concession 6	NB – LTR	A (0.21)	6	1	E (0.97)	50	15	300+
(Roundabout)	SB – LTR	B (0.43)	11	2	A (0.26)	7	1	580
Brock Street W/	EB – L	A (0.03)	9	1	A (0.09)	9	2	95
Centre Road	EB – T	A (0.25)	0	0	A (0.31)	0	0	95
(Unsignalized)	WB – TR	A (0.29)	0	0	A (0.30)	0	0	95
(Offsignalizeu)	SB – TR	D (0.58)	26	28	D (0.51)	34	21	185
Concession 6/	WB – LR	B (0.12)	12	3	B (0.16)	15	5	75
Bolton Drive	NB – TR	A (0.09)	0	0	A (0.30)	0	0	580
(Unsignalized)	SB – TL	A (0.01)	1	0	A (0.02)	1	0	300+
Bolton Drive/	EB – LTR	A (0.27)	10		A (0.16)	9		100
Centre Road/	WB – LTR	A (0.08)	9		A (0.08)	8		200
North Street	NB – LTR	A (0.15)	9		A (0.29)	9		135
(Unsignalized)	SB – LTR	B (0.35)	10		A (0.22)	9		105
Bolton Drive/	EB – TR	A (0.14)	8		A (0.12)	8		150
Quaker Village Dr	WB – TL	A (0.14)	9		A (0.15)	8		240
(Unsignalized)	NB – LR	A (0.24)	9		A (0.15)	8		300
Centre Road/	WB – LR	B (0.11)	10	3	B (0.09)	11	2	300+
Oakside Drive	NB – TR	A (0.07)	0	0	A (0.13)	0	0	105
(Unsignalized)	SB – TL	A (0.00)	0	0	A (0.00)	0	0	300+
Centre Road/	EB – LTR	B (0.15)	10	4	B (0.09)	11	3	300+
Maplebrook Dr/	WB – LTR	B (0.08)	10	2	B (0.11)	12	3	300+
Intersection #3	NB – LTR	A (0.01)	2	0	A (0.03)	2	1	280
(Unsignalized)	SB – LTR	A (0.00)	1	0	A (0.01)	1	0	300+
Concession 6/	WB – LR	B (0.21)	11	6	B (0.16)	14	5	50
Intersection #1	NB – TR	A (0.08)	0	0	A (0.28)	0	0	150
(Unsignalized)	SB – TL	A (0.01)	0	0	A (0.03)	2	1	300+
Concession 6/	WB – LR	B (0.11)	10	3	B (0.09)	13	3	50
Intersection #2	NB – TR	A (0.06)	0	0	A (0.22)	0	0	225
(Unsignalized)	SB – TL	A (0.00)	0	0	A (0.02)	1	0	150

Based on the intersection capacity analysis, under the 2028 future total traffic conditions, all the intersections considered are expected to operate at acceptable levels of service. Forecasted queues can be accommodated with the current and future available storage, all v/c ratios are under 1.0 and the highest approach delays are expected to be approximately 50 seconds. Therefore, no improvements are required under this horizon year and to accommodate the proposed development.

The analysis also indicates that the proposed site intersections are expected to operate at acceptable levels of service with minimum delay and queues. The required lane configures and traffic controls for the proposed site intersections based on the intersection operational capacity analysis are:

- 4. Concession 6/Site Intersection #1:
 - a. A shared left/right lane for westbound direction;
 - b. A shared through/right lane for the northbound and a shared through/left lane for the southbound direction; and
 - c. Stop controlled on the westbound direction (minor approach)
- 5. Concession 6/Site Intersection #2:
 - a. A shared left/through/right lane for eastbound and westbound directions;



- b. A shared left/through/right lane for the northbound and southbound directions; and
- c. Stop controlled on the westbound direction (minor approach)
- 6. Centre Road/Site Intersection #3/Maplebrook Drive Extension:
 - a. A shared left/through/right lane for eastbound and westbound directions;
 - b. A shared left/through/right lane for the northbound and southbound directions; and
 - c. Stop controlled on the Maplebrook Drive Extension and Site Intersection #3

6.2. Active Transportation Assessment

Walking

As indicated, currently, there are some sidewalks located on one side of the street along Centre Road, Bolton Drive and Oakside Drive. Sidewalks are located on both sides of the street along Brock Street W east of Quaker Village Drive/S Balsam Street through the downtown area. Sidewalks are generally in good conditions.

Based on Nextrans' review and assessment of the proposed draft plan of subdivision for the proposed development, the following recommendations are provided:

- Sidewalks be provided at least on one side of all internal streets. If possible, sidewalks be provided on both sides of the main east-west streets, if appropriate;
- Sidewalk be provided on the west side of Centre Road from the northerly limit of the proposed development to connect with the existing sidewalk on Bolton Drive and Centre Road south of Bolton Drive;
- Sidewalk be provided on the east side of Concession 6 from the northerly limit of the proposed development to connect with the existing sidewalk on the south side of Bolton Drive; and
- Two sidewalk/cycling connections provided between Bolton Drive and proposed development to connect with the existing sidewalk on the south side of Bolton Drive and sidewalks on Quaker Village Drive.

Figure 13 illustrates the proposed internal and external sidewalk connections for the proposed development.

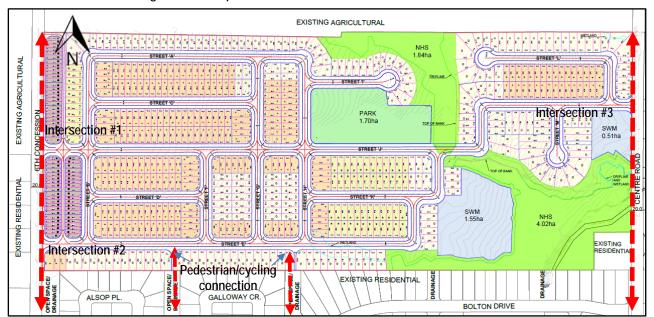


Figure 13 – Proposed Internal and External Sidewalk Connections



Cycling

As indicated, under the existing conditions, there are no dedicated bicycle lanes along the main streets in the area. However, there are some existing trails such as Maple Bridge Trail, Quaker Trail, Butternut Trail and S Balsam Trail in the area.

It is Nextrans' understanding that the Township of Uxbridge initiated the Active Transportation Plan project (ATP) and has retained WSP Canada Group Limited with Share the Road Cycling Coalition to support the development of the Active Transportation Plan in the Township. The project is scheduled to completed by the end of 2020. Based on the information provided on the Township's website and notice of study commencement, the objectives of the Project are:

- Expand educational and promotional initiatives that raise awareness of active transportation opportunities for people of all ages and abilities;
- Develop a continuous Township-wide active transportation network that connects to all communities within Uxbridge and builds upon existing trail systems found within the Township; and
- Collaborate with the Active Transportation Committee, the Trails Committee and the Accessibility Advisory
 Committee as well as local municipal stakeholders, residents and Durham Region staff to ensure the
 community's interests are addressed in the plan.

An external stakeholder workshop was held on Thursday November 5th, 2020 to get the public feedback on the project. Nextrans has reviewed and provided our own assessment and recommendations related to the proposed development based on the feedbacks and objectives of the ATP project. **Figure 14** illustrates the draft routes, as provided at the workshop.

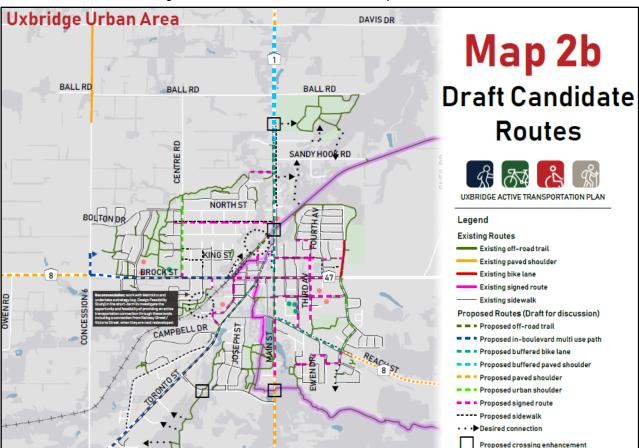


Figure 14 – Draft Candidate Active Transportation Routes



Nextrans is supportive of the proposed draft candidate routes and suggest that the project team for the proposed development continue to review and partipate in this process in order to implement the recommendations of ATP into the proposed development, where appropriate.

6.3. Transit Mode Assessment

Based on the information currently provided on the Township of Uxbridge website, Durham Region Transit offers three bus routes in the Township of Uxbridge: Route 601, Route 950 and Route 960. GO Transit also offers bus service and bus stops along Toronto Street in Uxbridge and Highway 47 in Goodwood.

However, based on the current information provided on Durham Region Transit website and system map, only bus route 905D is provided in the Township of Uxbridge at this time due to current COVID-19 pandemic situation.

As indicated, the proposed development is expected to generate 32 two-way none single-occupant-vehicle trips (8 inbound and 24 outbound) and 41 two-way single-occupant-vehicle trips (26 inbound and 15 outbound) during the AM and PM peak hours, respectively.

For the purposes of this assessment, it is assumed that both Route 905D and GO Bus are operating at a 30-minute headway during the peak hours, there would be four buses operating in the Township during the peak hours.

If it is assumed that all of the none single-occupant-vehicle trips are related to transit trips, the transit demand would be:

- 8 passengers/4 buses = 2 passengers per transit vehicle inbound and 24 passengers/4 buses = 6 passengers per transit vehicle outbound during the morning peak hour; and
- 26 passengers/4 buses = 7 passengers per transit vehicle inbound and 15 passengers/4 buses = 4 passengers per transit vehicle outbound during the afternoon peak hour.

The calculations above are conservative given that not all passengers will take the buses during the peak hour, but rather these demands will spread out during the peak period, i.e. between 6am and 9am, and 4pm and 7pm.

Based on this analysis, it is Nextrans' opinion that the transit demand from the proposed development is low and will not require any improvements on the existing transit network once it resumes normal operations after the COVID-19 pandemic.

7.0 INTERNAL SITE REVIEW

7.1. Truck Turning Movement Assessment

The proposed development will meet the Township of Uxbridge and Durham Region's standards and requirements for both internal and external intersection designs and street cross-sections. These requirements include but not limited to: turning radii, daylight triangles, sidewalk dimensions, grades, among other requirements.

Therefore, these designs will accommodate servicing vehicles such as garbage truck, emergency vehicles and delivery/moving vehicles.

AutoTURN software was used to generate the turning movement templates for servicing trucks (TAC-HSU) and illustrated in **Figure 21**. More detailed vehicle turning movements will be provided as part of the future application, if required.

7.2. Site Access Assessment

Two full moves intersections are proposed onto Concession 6, with the most southerly intersection is located approximately 225 m north of the Bolton Drive/Concession 6, with the northerly intersection is located approximately 150 north of the southerly intersection.



A full moves intersection is also proposed onto Centre Road, which is located approximately 400 m north of the Bolton Drive/Centre Road intersection and to be aligned with the future Maplebrook Drive extension located on the east side of Centre Road.

The intersection capacity analysis indicates that all three proposed development intersections are expected to operate at acceptable levels of service with minimum delays or queues. The recommended intersection configurations and traffic controls include:

- 1. Concession 6/Site Intersection #1:
 - a. A shared left/right lane for westbound direction;
 - b. A shared through/right lane for the northbound and a shared through/left lane for the southbound direction; and
 - c. Stop controlled on the westbound direction (minor approach)
- 2. Concession 6/Site Intersection #2:
 - a. A shared left/through/right lane for eastbound and westbound directions;
 - b. A shared left/through/right lane for the northbound and southbound directions; and
 - c. Stop controlled on the westbound direction (minor approach)
- 3. Centre Road/Site Intersection #3/Maplebrook Drive Extension:
 - a. A shared left/through/right lane for eastbound and westbound directions;
 - b. A shared left/through/right lane for the northbound and southbound directions; and
 - c. Stop controlled on the Maplebrook Drive Extension and Site Intersection #3

7.3. Sightlines

Nextrans has reviewed the existing conditions on both Centre Road and Concession 6 at the proposed development intersection access locations. This review indicates that both Centre Road and Concession 6 are relatively flat and no horizon curves at the proposed development intersection locations. These conditions are similar to the existing Centre Road/Oakside Drive and Concession 6/Bolton Drive intersections.

Appropriate daylight triangles will be provided at the proposed intersections onto Centre Road and Concession 6, as per the Township and the Region requirements.

7.4. Left Turn Warrant Analysis

The Ontario Ministry of Transportation (MTO) Left Turn Warrant analysis for at grade intersection was conducted to examine the need for exclusive left turns on Centre Road and Concession 6 at the proposed site intersections.

Centre Road/Maplebrook Drive Extension/Site Intersection #3

Figures 15 and 16 illustrate the MTO warrant guidelines for Centre Road/Maplebrook Drive Extension/Intersection #3.

- 50 km/ posted speed limit, assumed 60 km/h design speed
- Southbound: 14% (3 left turns x 100% / 28 advancing traffic volumes = 11% AM and 9 left turns x 100% / 63 advancing traffic volumes = 14% PM)
- Northbound: 49% (12 left turns x 100% / 30 advancing traffic volumes = 40% AM and 36 left turns x 100% / 73 advancing traffic volumes = 49% PM)



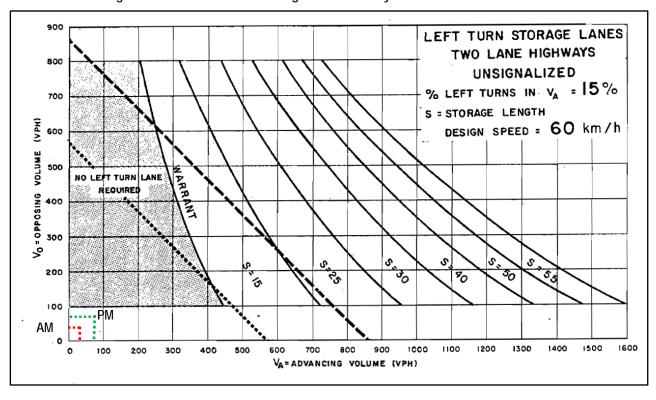
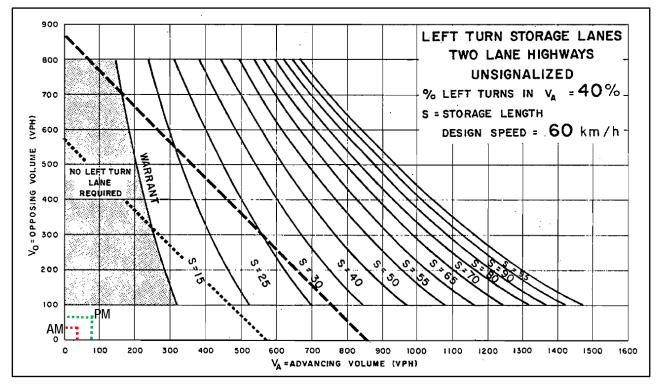


Figure 15 - MTO Left Turn Storage Warrant Analysis for Southbound Left Turn





Based on the analysis, it is anticipated that the southbound and northbound left turn lanes do not numerically meet the MTO warrant guidelines under the 2028 horizon.



Concession 6/Intersection #2

Figure 17 illustrates the MTO warrant guidelines for Concession 6/Intersection #2.

- 60 km/ posted speed limit, assumed 70 km/h design speed
- Southbound: 9% (17 left turns x 100% / 235 advancing traffic volumes = 7% AM and 15 left turns x 100% / 172 advancing traffic volumes = 9% PM)

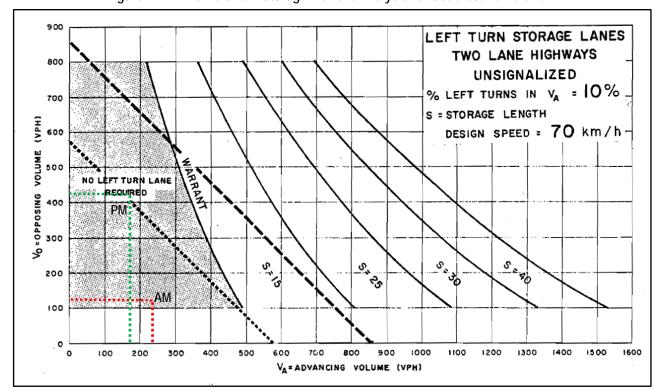


Figure 17 - MTO Left Turn Storage Warrant Analysis for Southbound Left Turn

Based on the analysis, it is anticipated that the southbound left turn lane does not numerically meet the MTO warrant quidelines under the 2028 horizon.

Concession 6/Intersection #1

Figure 18 illustrates the MTO warrant guidelines for Concession 6/Intersection #1.

- 60 km/ posted speed limit, assumed 70 km/h design speed
- Southbound: 21% (8 left turns x 100% / 122 advancing traffic volumes = 7% AM and 27 left turns x 100% / 127 advancing traffic volumes = 21% PM)

Based on the analysis, it is anticipated that the southbound left turn lane does not numerically meet the MTO warrant quidelines under the 2028 horizon.



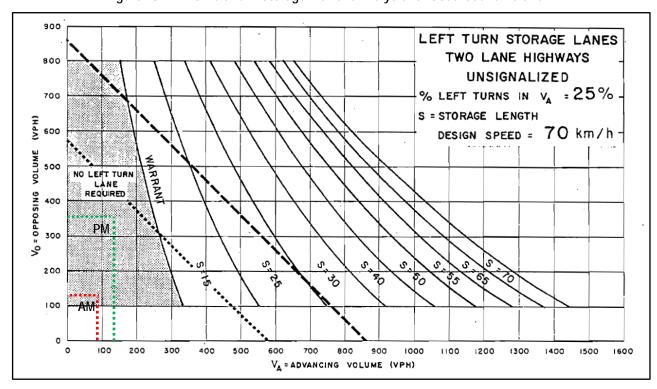


Figure 18 - MTO Left Turn Storage Warrant Analysis for Southbound Left Turn

7.5. Signal Warrant Analysis for the Brock Street W/Centre Road Intersection

As requested by Durham Region, Nextrans has conducted a traffic signal warrant analysis for the Brock Street W/Centre Road intersection based on the future forecast volumes. The traffic signal warrant analysis as outlined in **Appendix G** indicates that minimum vehicular volume and delay to cross traffic for major street approaches may be warranted (Justification 7 of the Ontario Traffic Manual Book 12), however, the minor street approaches and traffic crossing major street are not warranted because of the low turning traffic volumes and this is a T-intersection.

7.6. Durham Vision Zero Strategic Road Safety Action Plan

Vision Zero

Vision Zero is a Swedish program that successfully halved traffic death using failsafe systems of road design, vehicle design and speed control. It aims to remove the possibility of any death or serious injury from transportation and road systems. No loss of life resulting from a motor vehicle collision is acceptable.

The context of Vision Zero in Canada, Durham Region and the Town of Uxbridge is described below:

- Canada https://visionzero.ca
 - o Spreads awareness about Vision Zero and includes guides/toolkits for implementation
- Durham Region https://www.durham.ca/en/living-here/vision-zero.aspx
 - o Durham has a Strategic Road Safety Action Plan that is fundamentally based on Vision Zero
 - Goal: 10% reduction of fatal and injury collisions within five years (2019-2023).
 - Lists possible (but generalized) actions to meet this goal for intersections, pedestrians, cyclists, distracted driving, and more
 - Article from Nov. 2020 says that number of collisions are down but fatalities are unchanged



In Uxbridge

- o On Apr. 6, 2020, Durham Region staff presented Vision Zero and various stats to the Uxbridge General Purpose and Administration Committee. This was Agenda Item 10.1 and moved by Councillor Snooks
- Concession Rd 6 and Davis Dr has the greatest collision frequency in Uxbridge
 - Presentation does not include any improvements for this intersection strangely
- Region proposing many Regional projects to implement Red Light Cameras, Roundabouts, and more in Uxbridge.
 - Goodwood and Downtown Uxbridge are the main areas receiving these improvements
 - The only nearby road improvement project is a roundabout at Reg. Hwy 47 (Toronto St) at Concession Rd 6 (construction in 2025, pending Region approval)
- O Uxbridge Council endorsed the meeting minutes from Apr. 6 (as Item 9.1) on Apr. 27. Moved by Councillor Popp and seconded by Councillor Snooks.

Vision Zero for the Proposed Development

As requested by the Durham Region staff, Nextrans will also recommend the proposed development to include the following requirements from Durham's Vision Zero Strategic Road Safety Action Plan. These requirements were based on Nextrans' review of Durham Region's Strategic Road Safety Action Plan (<u>Durham Region staff presented Vision Zero</u>). These requirements are based on the context of the proposed development.

Roundabout

 It is anticipated that the forecast traffic volumes at the proposed internal intersections and external intersections are very low, which will not meet the warrant for roundabout treatments. It is Nextrans' opinion that the stop signs and associated intersection treatment recommended in this Study would address these concerns

• Traffic Signal Treatments

 As the traffic signals are not warranted for the proposed intersections as part of the proposed development, no traffic signal treatments are required at this time

Tighter Curb Radii

- o Provide smaller turning radii at all internal and external intersections to increase pedestrian safety by minimizing the crossing distances, increasing pedestrian visibility, and decreasing vehicle turning speed
- o The proposed locations with recommended tight turn radii are illustrated in Figure 19 of this Study
- Roadway Design to Discourage Speeding
 - o Minimize the lane widths, edge lines, curb and gutter for all internal roads, as well as on Concession Road 6 and Centre Road along the frontage of the proposed development to discourage aggressive driving
- Lane Widening for Truck
 - o It is recommended that the proposed development to maintain a minimum of 1.2 m to 1.5 m paved shoulder width along the frontage of the subject site along Concession Road 6 and Centre Road.
- Flexible In-Road Warning Signs
 - o Given that Nextrans recommended to minimize the lane widths for all internal roads, installation of flexible in-road warning signs to guide traffic into a specific lane are not required. It is recommended that the Region and the Town consider install these flexible in-road warning signs for Concession Road 6, Centre Road, Bolton Drive, Brock Street W and Quaker Village Drive in the area



Transverse Rumble Strips

o It is Nextrans' opinion that this is not required for the proposed development. It is recommended that the Town consider this treatment for Centre Road southbound and northbound north and south of Bolton Drive/North Street

Zebra Crosswalks

o It is recommended that raised and enhanced crosswalk markings (ladder/zebra) at all all-way stop intersections and approaches with stop signs, as well as at all pedestrian crossing locations

Crossrides

o Given that there is no update yet on the WSP's Active Transportation Plan at the time of preparation of this Transportation Impact Study, Nextrans recommends that these requirements be reviewed as part of the detailed and engineering design at that time. It is recommended that the proposed development protect for this treatment in the future.

Paved Shoulders

o It is recommended that the proposed development to maintain a minimum of 1.2 m to 1.5 m paved shoulder width along the frontage of the subject site along Concession Road 6 and Centre Road.

It should be noted that since this Transportation Impact Study is prepared to support the proposed Official Plan and Zoning By-law Amendment applications, this Study is not a detailed designed or engineering study. Therefore, most of these requirements will be addressed through the detailed site design and engineering submission at the subsequent stage of the proposed development.

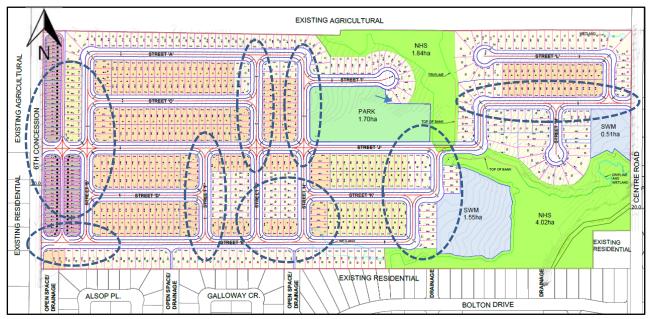


Figure 19 – Recommended Tight Radii at the Proposed Intersection

7.7. Internal Intersection Lane Configurations and Traffic Control Types

Based on the external intersection operational capacity analysis findings, site traffic generation and the review of ATP objective and directions, Nextrans has reviewed the internal intersection and street network based on these principles:

- Internal street network should accommodate all modes of transportation: walking, cycling and cars
- Internal intersection lane configurations should minimize crossing/walking distance for pedestrians and cyclists;
 and



Intersection traffic control types should take into consideration of the pedestrian and cyclist safety requirements.

To address the principles above, Nextrans identified and recommended the following requirements for internal streets and intersections:

- Sidewalks be provided at least on one side of all internal streets. If possible, provide sidewalks on both sides of the main east-west streets;
- All-way stops be provided at the main intersections with pedestrian crossings; and
- No exclusive turning lanes are required for traffic operation and capacity, as well as to minimize the crossing distance for pedestrians and cyclists.

Figure 20 illustrates the proposed traffic control types for internal intersections to accommodate pedestrians, cyclists and cars.

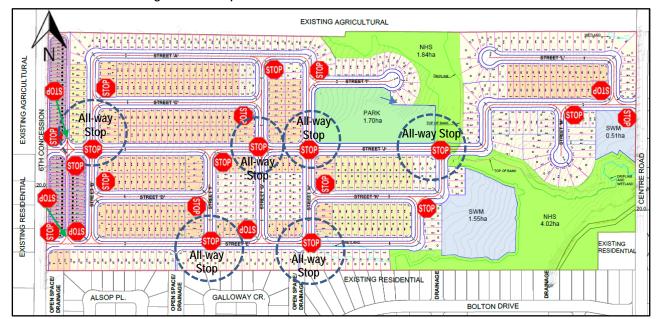


Figure 20 - Proposed Internal and External Sidewalk Connections

8.0 TRANSPORTATION DEMAND MANAGEMENT

Transportation Demand Management (TDM) is a co-ordinated series of actions aimed at maximizing the people moving capability of the transportation system. It is intended help reduce single-occupant auto use. Potential TDM measures may include but not limited to: TDM supportive land use, bicycle and pedestrian programs and facilities, public transit improvements, preferential treatments for buses and high occupancy vehicles (if applicable), ridesharing, and employee incentives.

It is Nextrans' understanding that the Township of Uxbridge initiated the Active Transportation Plan project (ATP) and has retained WSP Canada Group Limited with Share the Road Cycling Coalition to support the development of the Active Transportation Plan in the Township. The project is scheduled to completed by the end of 2020. Based on the information provided on the Township's website and notice of study commencement, the objectives of the Project are:

- Expand educational and promotional initiatives that raise awareness of active transportation opportunities for people of all ages and abilities;
- Develop a continuous Township-wide active transportation network that connects to all communities within Uxbridge and builds upon existing trail systems found within the Township; and



Collaborate with the Active Transportation Committee, the Trails Committee and the Accessibility Advisory
Committee as well as local municipal stakeholders, residents and Durham Region staff to ensure the
community's interests are addressed in the plan.

Based on the review of the context of the proposed development in relation to the TDM requirements in the Township and the Region's Guidelines, as well as the information noted above, the following TDM measures and incentives are recommended for the proposed development:

Active Transportation:

- o Sidewalks be provided at least on one side of all internal streets. If possible, sidewalks be provided on both sides of the main east-west streets, if appropriate;
- Sidewalk be provided on the west side of Centre Road from the northerly limit of the proposed development to connect with the existing sidewalk on Bolton Drive and Centre Road south of Bolton Drive;
- o Sidewalk be provided on the east side of Concession 6 from the northerly limit of the proposed development to connect with the existing sidewalk on the south side of Bolton Drive; and
- Two sidewalk/cycling connections provided between Bolton Drive and proposed development to connect with the existing sidewalk on the south side of Bolton Drive and sidewalks on Quaker Village Drive.

Information Package:

- Provide information package all residential units. The information package includes: Durham Region Transit schedules and maps, GO Bus schedule and maps, community major destinations and cycling maps;
- The information package can be a form of a letter that includes all website links (QR codes) to the available public transit, community amenities and cycling resources

9.0 CONCLUSIONS / FINDINGS

9.1. Study Conclusions

The findings and conclusions of the analysis are as follows:

- The proposed development is expected to generate:
 - o 393 total two-way trips (97 inbound and 296 outbound) and 515 total two-way trips (325 inbound and 190 outbound) during the AM and PM peak hours, respectively;
 - o 361 two-way auto trips (89 inbound and 272 outbound) and 474 two-way auto trips (299 inbound and 175 outbound) during the AM and PM peak hours, respectively; and
 - 32 two-way none single-occupant-vehicle trips (8 inbound and 24 outbound) and 41 two-way single-occupant-vehicle trips (26 inbound and 15 outbound) during the AM and PM peak hours, respectively.
- The intersection capacity analysis indicates that under existing, future background and future total conditions, all the intersections considered in the Study are expected to operate at acceptable levels of service. No physical improvements to the area road network and intersections are required for this horizon year.
- The analysis indicates that no improvements are required to the existing active transportation and Durham Region Transit/GO Bus service in the area to accommodate the proposed development and this horizon year.



- The intersection capacity analysis indicates that all three proposed development intersections are expected to operate at acceptable levels of service with minimum delays or queues. The recommended intersection configurations and traffic controls include:
 - o Concession 6/Site Intersection #1:
 - A shared left/right lane for westbound direction;
 - A shared through/right lane for the northbound and a shared through/left lane for the southbound direction; and
 - Stop controlled on the westbound direction (minor approach)
 - Concession 6/Site Intersection #2:
 - A shared left/through/right lane for eastbound and westbound directions;
 - A shared left/through/right lane for the northbound and southbound directions; and
 - Stop controlled on the westbound direction (minor approach)
 - Centre Road/Site Intersection #3/Maplebrook Drive Extension:
 - A shared left/through/right lane for eastbound and westbound directions;
 - A shared left/through/right lane for the northbound and southbound directions; and
 - Stop controlled on the Maplebrook Drive Extension and Site Intersection #3
- To accommodate pedestrians and cyclists to/from the proposed development, the following recommendations are provided:
 - O Sidewalks be provided at least on one side of all internal streets. If possible, sidewalks be provided on both sides of the main east-west streets, if appropriate;
 - Sidewalk be provided on the west side of Centre Road from the northerly limit of the proposed development to connect with the existing sidewalk on Bolton Drive and Centre Road south of Bolton Drive:
 - Sidewalk be provided on the east side of Concession 6 from the northerly limit of the proposed development to connect with the existing sidewalk on the south side of Bolton Drive;
 - Two sidewalk/cycling connections provided between Bolton Drive and proposed development to connect with the existing sidewalk on the south side of Bolton Drive and sidewalks on Quaker Village Drive;
 - o All-way stops must be provided at the main pedestrian crossings; and
 - No exclusive turning lanes are required for traffic operation and capacity, as well as to minimize the crossing distance for pedestrians and cyclists.

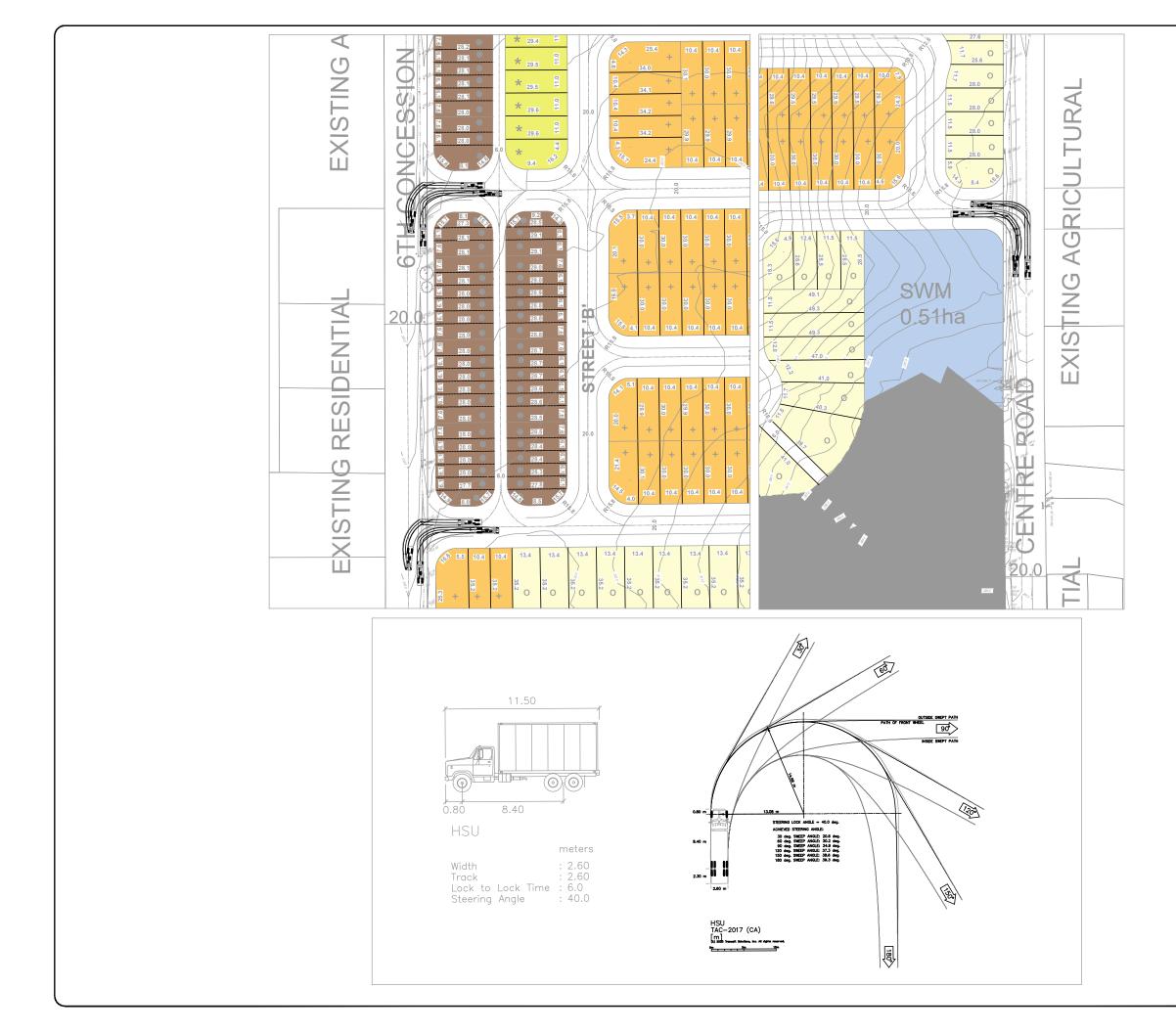
9.2. Study Recommendations

Based on the Study findings and conclusions, the following recommendations are provided:

- The proposed development implements the TDM measures and incentives identified in this report to support
 active transportation and transit and to reduce the numbers of single-occupant-vehicle trips to and from the
 proposed development
- The proposed development implements Durham's Vision Zero Strategic Road Safety Action Plan requirements
 for the internal roads and intersections, as well as the external intersections onto Concession Road 6 and Centre
 Road, as recommended in Sections 7.6 and 7.7 of this Study



- The proposed development provides the recommended internal and external active transportation network
- The proposed development provides the recommended traffic control types for the internal and external intersections
- No additional physical improvements for the area road network and intersection to accommodate the proposed development under the future total conditions for the horizon year considered



KEY PLAN BENCHMARK

NO REVISION DATE B

nexirans
consulting Engineers
520 Industrial Parkway South, Suite 201
Aurora, Ontario L 46 6V/8
161: 305-302-3633

PROJECT NAME

RESIDENTIAL DEVELOPMENT 7370 Centre Road (TOWNSHIP OF UXBRIDGE)

RAWING TITLE:

AutoTURN Analysis (HSU TAC-2017)

DATE: December 11, 2020
PROJECT NO.
NT-20-058
DRAWING NO.
Figure 21

Appendix A

Terms of Reference and Correspondence

From: Glyn Reedman < Glyn.Reedman@Durham.ca>

Sent: May 12, 2020 4:11 PM
To: Andy Bilaweijan

Subject: FW: 7370 Centre Road Terms of Reference

Attachments: Transportation Impact Study Guidelines OCT 2011 Final.pdf; Synchro Guidelines - Section 9.pdf

Hi Andy.

Thank you for submitting the Terms of Reference for our review.

The challenge with preparing a TIS at this time is the inability to obtain up-to-date base traffic data. You state that traffic counts will be undertaken as part of the TIS, however this is not going to give accurate base data due to the Covid-19 restrictions currently in place.

We would require the intersections of Regional Road 8 / Centre Road and Regional Road 8 / Concession 6 (roundabout) to be included in the scope of the study. There is a traffic count in 2018 at Regional Road 8 / Concession 6, however the latest traffic count at Regional Road 8 / Centre Road is 2013. For the purposes of this study, we would consider using these counts with appropriate background traffic growth applied, in the absence of being able to take new traffic counts. We are not sure what data may be available on the Township Roads for the intersections included in your study area and this is going to require you to give this further consideration and discuss with the Township before an acceptable Terms of Reference can be agreed. We suggest that you consider what data is available and resubmit your Terms of Reference.

Below are our general comments on the scope of the study which you will need also to take account of.

- 1. Your study should comply with the Region's Traffic Impact Study Guidelines, including the requirements for Synchro analysis. I attach both for your information.
- 2. We would like you to include Regional intersections noted above. The report should include a signal warrant analysis of the Regional Road 8 / Centre Road intersection.
- 3. We generally agree with the use of a 5-year horizon from full build out. Please use an achievable / realistic full build out year.
- 4. We agree with the methodology for trip generation, distribution and background traffic growth. The growth rate and background developments should be agreed before the study is submitted.
- 5. Typically existing conditions should be noted by way of a site visit and any observations noted in the TIS. Due to Covid-19 restrictions we will not require this in this case, if the TIS is to be completed during the restrictions.
- 6. The most current intersection turning movement counts on the Regional roads (ATR counts and AADT data) as noted above, can be downloaded from our web site through the interactive <u>traffic counts map</u>. Traffic counts taken or used on Township of Uxbridge roads will need to be confirmed with the Township.
- As per the Region's TIS Guidelines, please include transit, active transportation and TDM discussions in the TIS.
 Recommendations should include infrastructure, network and program improvements to support non-auto
 travel.

Glyn Reedman Project Coordinator Transportation Infrastructure 905-668-7711 Ext 3476

From: Andy Bilawejian <andy@nextrans.ca>

Sent: May 6, 2020 10:25 AM

To: Doug Robertson < <u>Doug.Robertson@Durham.ca</u>> **Subject:** 7370 Centre Road Terms of Reference

Good morning Doug,

Please see attached Terms of Reference for 7370 Centre Road in the Town of Uxbridge.

Please advise whether the attached ToR is acceptable, or if you have any comments.

Thanks,

Andy Bilawejian, B.Eng., EIT

Transportation Analyst

o: 905-503-2563 ext. 209

c: 416-358-2348 e: andy@nextrans.ca w: www.nextrans.ca

NexTrans Consulting Engineers
A Division of NextEng Consulting Group Inc.
520 Industrial Parkway South, Suite 201
Aurora ON L4G 6W8

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nexirans
CONSULTING ENGINEERS

Phone: 905-503-2563 www.nextrans.ca

NextEng Consulting Group Inc.

Terms of Reference

Reviewed on behalf of the Twp of Uxbridge

AECOM PN 60240627

Pre-Con follow up Date: Nov 9, 2020

By: Peter Middaugh, P.Eng.

To: Peter Middaugh

From: Andy Bilawejian, Transportation Analyst, Nextrans Consulting Engineers

Date: April 4, 2019

Re: 7370 Centre Street, Uxbridge, Residential Subdivision – TOR for Traffic Impact Study

These terms of reference have been prepared to outline (for the Town's review and approval) the intended scope of work for a transportation impact and parking justification study for a proposed residential subdivision consisting of 607 dwelling units. The subject site is located north of Bolton Drive between Centre Street and Concession Road 6 in the Town of Uxbridge.

Introduction

The report introduction will include:

- 1. Description of site location
- 2. Description of nature of application
- 3. Description of proposed development and land use
- 4. Proposed study area

Existing Traffic Assessment

The existing conditions within the study area will be summarized and documented. This will include, but not limited to:

- A description of key roads and intersections (lanes, speed limits)
- Identifying forms of traffic control, lane configurations, turning restrictions
- Identifying pedestrian and cycling facilities
- Noting the location of adjacent driveways and access points
- Identifying other traffic generators in the vicinity of the site

Turning movement counts will be collected during weekday AM (7am-10am), weekday PM (4pm-7pm) peak periods at the following study area intersections:

- Bolton Drive and Concession Road 6
- Bolton Drive / North Street and Centre Road
- Bolton Drive and Quaker Village Drive
- Centre Road and Oakside Drive

Centre Street / Brock St W (Regional Road 8)

Once existing traffic volumes have been collected, we will prepare a baseline model of existing traffic operations at the study area intersections using Synchro v.10 analysis for the identified critical time periods (weekday AM and PM peak hours). The existing analysis will include levels of service, volume to capacity ratios, and queuing at the key study intersections.

Future Background Traffic Assessment

Future Background consists of background growth and other background development traffic. We will obtain historic AADT records and estimate a background growth rate for the assumed full build-out year for the proposed development along with a 5-year time horizon period thereafter.

We do understand that there is and may be further redevelopment applications, as such traffic generation associated with those developments will be included in our analysis to reflect our horizon year assessment.

Operational deficiencies as a result of future forecasted traffic volumes will be identified and mitigative measures will be proposed and documented in the final report.

Site Traffic Assessment

The weekday AM and PM peak hour traffic to be generated by the proposed development will be estimated based on information published in the *Trip Generation*, 10th Edition, by the Institute of Transportation Engineers (ITE).

The directional trip distribution and assignment for traffic approaching and departing the site will be determined based upon existing traffic patterns and Transportation Tomorrow Survey (TTS) 2016 data.

Future Total Traffic Assessment

Future total traffic consists of future background plus site traffic. Operational deficiencies as a result of site traffic will be identified and mitigative measures will be proposed and documented in the final report. We will develop and recommend appropriate intersection controls and geometric improvements for all key intersections as well as determine the appropriateness of the proposed site access location(s) and the lane requirements at these new locations.

Parking / On Site Circulation and Site Access Review

- Review the available parking to determine whether the proposed parking supply is sufficient to accommodate the parking demand of the proposed site and meets current bylaw requirements.
- We will review and provide comment on the most recent site plan with respect to the functionality of the internal vehicular circulation to facilitate vehicle maneuvering, loading, servicing, parking and pick-up / drop-off activities.
- Using Auto TURN, we will confirm the turning radius requirements and site circulation for passenger and heavy vehicles.
- Determine the appropriateness of access location and ensure adequate connections to main corridors are provided.
- Assign appropriate internal signage to site plan.

Coordinate assessment and review with the planned Mason Homes development on the east side of Centre Street. The planned location of the intersections on Centre Street for both the 7370 Centre Street plan and the Mason Homes plan need to be coordinated and details of such coordination and decisioning captured in the report.

Transit and Transportation Demand Management Plan

A review of the existing and future transit availability in the area and recommendations shall be made to ensure acceptable walking distances are proposed to the subject lands. Transit routes, service frequencies, and stations will be identified in the study area.



Vision Zero – A Strategic **Road Safety Action Plan for Durham Region**









Vision and Goal

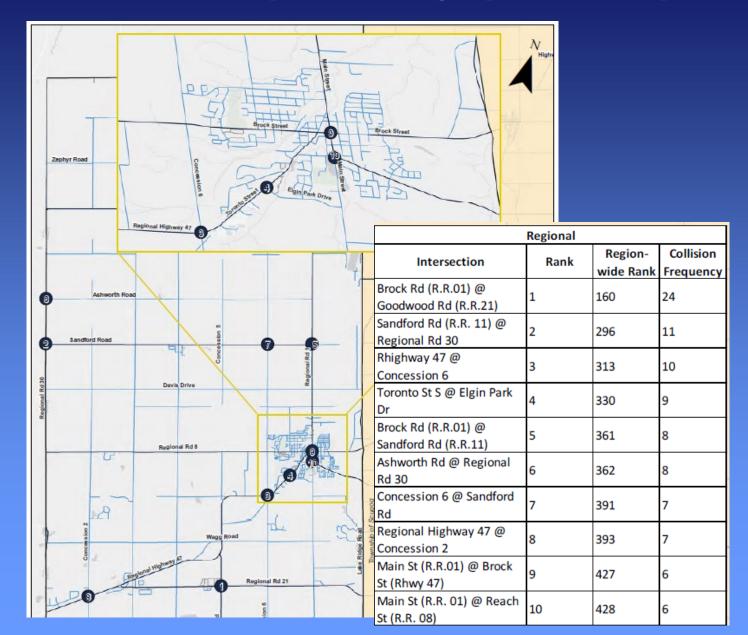
Vision

Zero people killed or injured across all modes of transportation.

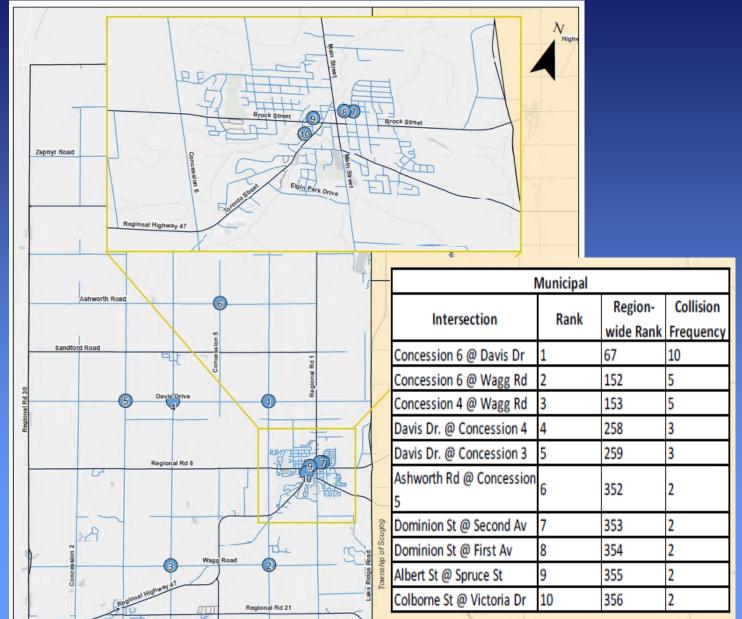
Goal

Minimum 10% reduction in fatal and injury collisions over a five year period.

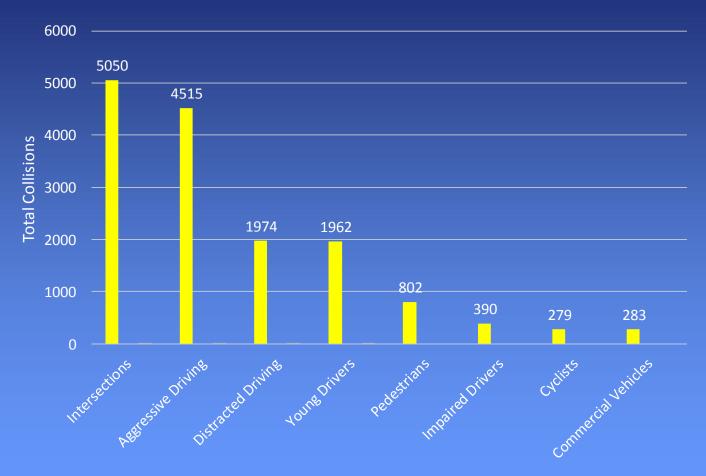
Top 10 Highest Collision Frequency Regional Intersections in Township of Uxbridge (2014-2018)



Top 10 Highest Collision Frequency Municipal Intersections in Township of Uxbridge (2014-2018)



The Data



Five Year Total Fatal and Injury Collisions by Emphasis Area (2012-2016)
Regional and Local Municipal Roads Combined

Intersections

- Red Light Cameras
 - Reg. Rd. 23 (Lake Ridge Rd) at Reg. Rd. 21 (Goodwood Rd)
- Roundabouts
 - Reg. Rd. 23 (Lake Ridge Rd) at Davis Drive
 - Reg. Rd. 21 (Goodwood Rd) at Brock Rd (construction 2024*)
 - Reg. Hwy 47 (Toronto St) at Concession Rd 6 (construction 2025*)
 - Reg. Rd. 21 (Goodwood Rd) at Concession Rd 6

Intersections

- Protected Only Left Turn Phasing
- Physical Improvements (turning radii, offset left turn lanes, right turn lanes)
- DVZ Policy (Signal Warrants, Roundabouts)
- Traffic Control Signals
 - Reg. Rd. 1 (Main St N) at Toronto St N
 - Reg. Rd. 8 at Reg. Rd. 30

Aggressive Driving

- Automated Speed Enforcement
 - Reg. Hwy 47 Goodwood
 - Reg. Rd. 11 Sandford
- Radar Feedback Board Program
 - Reg. Hwy 47 (Brock Street East), east of Nelkydd Lane
 - Reg. Rd. 1 (Main Street), Fields of Uxbridge
 - Reg. Rd. 13, east of Leaskdale
 - Permanent Locations: Goodwood, Chalk Lake
- Line Marking Enhancements

Aggressive Driving

- DVZ Policy (Speeds, Community Safety Zone)
- Design to Discourage Speeding
 - Leaskdale, Toronto Street
- County Curve Program
 - Reg. Rd. 30 at Reg. Rd 39
 - Reg. Rd. 39 at Meyers Rd
 - Reg. Rd. 13 at Acton Rd
 - Reg. Rd. 23 at Townline Uxbridge-Brock (Vallentyne)

Distracted Driving

- Education and Enforcement
- Safety Edge

Young Drivers (16-25)

- Education Campaigns
 - Weed out the Risk
 - Distracted Walking Programs
 - PARTY Program
 - National Teen Driver Safety Week

Pedestrians

- Pedestrian Crossover Installations
 - Toronto Street at Colborne Street (2020)
 - Brock Street West at Centre Road (2022/23)
- Midblock Pedestrian Signal
 - Main Street North, north of Colby Road (2021)
- Intersection Pedestrian Signals
 - Reg. Hwy 47 at Concession Rd 3 (Front St) (2021)
 - Brock Street West at Victoria St/Railway St (2021)

Pedestrians

- Pedestrian Signal Timing Adjustments
- Pedestrian Countdown Signals
- Leading Pedestrian Interval Pilot Project
- Refuge Islands
- Zebra Markings

Impaired Drivers

Education and Enforcement

Cyclists

- Paved Shoulders on Rural Roads
- Midblock Trail Crossing Study
- Cross-rides at Intersections
- Cyclist Detection at Traffic Signals

Commercial Vehicles

- Designated Truck Routes
- Additional Enforcement Efforts

Preliminary ASE Data

- Over 10000 fines issued since September
- Speeds reduced by approximately 30 percent
- Currently there two ASE Sites in Uxbridge:

Sandford Rd (Reg. Rd. 11)

- 3rd Rotation Dec 2020

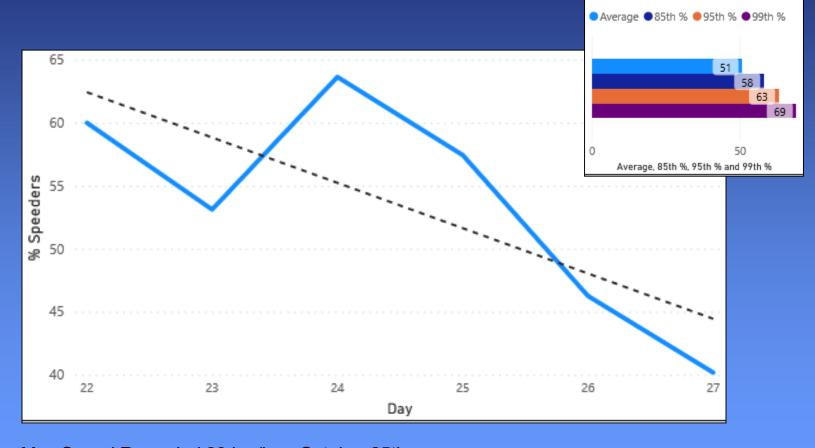
• Goodwood Rd (Reg. Hwy 47

- 2nd Rotation Oct 23, 2020



Preliminary ASE Data (Goodwood)

Speed Data



Max Speed Recorded 89 km/hr - October 25th

AUTOMATED SPEED ENFORCEMENT SYSTEM OFFENCE NOTICE AVIS D'INFRACTION - SYSTÈME DE CONTRÔLE AUTOMATISÉ DE LA VITESSE

Form / Formulaire 6.2

ONTARIO COURT OF JUSTICE COUR DE JUSTICE DE L'ONTARIO

THE REGIONAL MUNICIPALITY OF DURHAM,

825 CONLIN RD

(Address / Adresse): WHITBY, ON, L1N6A3

being the owner of a motor vehicle displaying propriétaire d'un véhicule automobile muni de la

Ontario number plate AP48777 plaque d'immatriculation de l'Ontario are charged with the offence of speeding

êtes accusé(e) de l'infraction d'excès de vitesse September , 2020 iour de

Simcoe St. North, Northbound North of Selleck Lane on

(street/road / rue/chemin) a community safety zone in the

une zone de sécurité communautaire dans le/la

Regional Municipality of Durham, City of Oshawa (municipality / municipalité)

as shown in the digitized image set forth in this notice, contrary to section 128 and pursuant to section 207 of the Highway Traffic Act.

comme il est indiqué dans les images numérisées présentées dans le présent avis, contrairement à l'article 128 et conformément à l'article 207 du Code de la

The photograph taken by the automated speed enforcement system shows the La photographie prise par le système de contrôle automatisé de la vitesse montre

motor vehicle travelling at 68

kilometres per hour in a posted 50 kilomètres à l'heure dans une zone où la vitesse est limitée à

kilometre per hour zone. kilomètres à l'heure.

I believe and cartify that the above offence has been committed Je crois et l'atteste que l'infraction mentionnée ci-dessus a été commise.

Signature of the Officer issuing this notice Signature de l'agent qui a délivré l'avis d'infraction

Issuing Officer Number: 00111

Matricule de l'agent qui a délivré l'avis d'infraction :

Date of Deemed Service: October 6, 2020

Date de signification présumée

PLEASE NOTE: Section 207 of the Highway Traffic Act provides that you, as the owner, are liable for this offence even if you were not the driver at the time subject to limited exceptions. Neither dement points nor a driver's licence suspension will result from your conviction for this offence. The provincial offences officer has certified that the automated speed enforcement system used in the detection of this offence complied with section 2 of Ontario Regulation 398/19, was in proper working order at the time that the photograph was recorded by that system and was used in an area authorized by law. A certified photograph will be tendered in evidence at your trial. You must apply to the justice at trial if you wish to compel the attendance of the Provincial Offences Officer who issued the certificate of offence or who certified the photograph to be tendered at your trial.

Set Fine Amende fixée \$ 90.00 \$

Total Payable: Montant total exigible (includes set fine, applicable victim fine surcharge and

suramende compensatoire pour l'aide aux victimes applicable et les frais)

costs) (comprend l'amende fixée, la

\$ 115.00 \$

REMARQUE : L'article 207 du Code de la route prévoit que vous-même, à titre de propriétaire, êtes responsable de cette infraction même si yous ne conduisiez pas le véhicule à ce moment-là. sous réserve d'exceptions restreintes. Une déclaration de culpabilité pour cette infraction ne mênera pas à l'inscription de points d'inaptitude dans votre dossier ou à la suspension de votre permis de conduire. L'agent des infractions provinciales a attesté que le système de contrôle automatisé de la vitesse utilisé pour détecter cette infraction était conforme à l'article 2 du Règlement de l'Ontario 398/19, qu'il fonctionnait correctement au moment de la prise de la photographie et qu'il était utilisé dans un secteur autorisé par la loi. Une photographie certifiée sera présentée en preuve à votre procès. Vous devez vous adresser au juge du procès si vous désirez obtenir la comparution de l'agent des infractions provinciales qui a délivré le

procès-verbal d'infraction ou qui a certifié la photographie qui sera

présentée en preuve lors de votre procés.





Provincial Offences Act / Loi sur les infractions provinciales

2860-994-20-G0020736-00

O. Reg. / Rèal. de l'Ont. 108/11





FORWARD YOUR RETURN SLIP IN THE SELF-ADDRESSED ENVELOPE PROVIDED. I ENVOYEZ VOTRE BORDERAU DANS L'ENVELOPPE-RÉPONSE FOURNIE.

Location: Simcoe St. North, Northbound North of Selleck Lane Date: 15/09/2020 Time: 13:24 Posted Speed Limit: 50 KPH Motor Vehicle Speed: 68 KPH

Lane: 2 System ID: 115996-0012

Preliminary RLC Results

- All twelve Red Light Cameras installed
- Ten of twelve Red Light Cameras full operational issuing charges
- One site in Uxbridge at Lake Ridge and Goodwood



RED LIGHT CAMERA SYSTEM OFFENCE NOTICE AVIS D'INFRACTION – SYSTÈME PHOTOGRAPHIQUE RELIÉ AUX FEUX ROUGES

Form 5 Provincial Offences Act, O. Reg 108/11 Formulaire 5, Loi sur les infractions provinciales, Règl. de l'Ont. 108/11

ONTARIO COURT OF JUSTICE COUR DE JUSTICE DE L'ONTARIO

You/Vous (Name/Nom): (Address/Adresse):



being the owner of a motor vehicle displaying étant le propriétaire d'un véhicule automobile muni de la

Ontario number plate

plaque d'immatriculation de l'Ontario

are charged with the offence of failing to stop at a red light étes accusé(e) de l'infraction d'omettre de s'arrêter à un feu rouge

on the

04 day of September, 2018 at 12:44 PM jour de

à l'intersection de

at the intersection of Bayview Ave. and Crosby Ave. / Redstone Rd.

(location / endroit)

in the dans le/la Regional Municipality of York

(municipality / municipalité)

as shown in the digitized images set forth in this notice, contrary to subsection 144(18.1) and pursuant to section 207 of the Highway Traffic Act. comme il est indiqué dans les images numérisées présentées dans cet avis, contrairement au paragraphe 144(18.1) et à l'article 207 du Code de la route.

The photographs taken by the red light camera system show the vehicle approaching the intersection, at which time the signal had displayed red for Les photographies prises par le système photographique relié aux feux rouges montrent le véhicule qui s'approche de l'intersection, au moment où le feu était rouge depuis

> 001.2 seconds and that vehicle proceeded through the intersection when 001.2 secondes et que le véhicule a franchi l'intersection alors que

the light had been red for

001.9 seconds.

le feu était rouge depuis

001.9 secondes.

I believe and certify that the above offence has been committed. Je crois et atteste que l'infraction susmentionnée a été commise.

Signature of Officer issuing this notice: Signature de l'agent qui délivre le présent avis:

Issuing Officer Number: 27 Numéro de l'agent qui délivre le présent avis:

Date of Deemed Service: September 19, 2018

Date de signification présumée:

4961-997-18-30240195-00

OFFENCE NO. / Nº D'AVIS D'INFRACTION







Next Steps

- Implementation of Countermeasures
- Continued Partnership and Collaboration with Durham Regional Police Service and Local Area Municipalities
- Ongoing monitoring and evaluation of progress towards zero.

durham.ca/DurhamVisionZero

TOWNSHIP OF UXBRIDGE

ACTIVE TRANSPORTATION PLAN









EXTERNAL STAKEHOLDER WORKSHOP

Thursday November 5th, 2020







Agenda

1. Introductions 10 min

2. Project Overview 20 min

- Study Process
- ATP Part 1 Engagement Overview
- ATP Part 1 Outcomes
- ATP Part 2 Considerations
- 3. Interactive Activities
 - Draft Candidate Network
 - Programs and Outreach
- 4. Wrap-up and Next Steps 5 min









60 min

Today's Objectives

- Gather input on a draft candidate network including any modifications to proposed routes and facility types.
- Gather input on network priorities that should be identified in the plan.
- 3. Prioritize education, encouragement and outreach initiatives.











PROJECT OVERVIEW











What is an ATP?

The Active Transportation Plan (ATP)

Is:

- Long-term vision
- Flexible document
- Community building asset
- Communication tool
- Implementation guide
- Support for existing plans

Is not:

- X Detailed or final design
- X Authority to construct
- Prescriptive
- **X** Requirement
- X Financial commitment









Study Process



Future Directions, Strategies and Actions

- Undertake a strengths, weaknesses, opportunities and threats (SWOT) analysis of active transportation in Uxbridge.
- Assess demand and potential for active transportation in Uxbridge, using available data from Statistics Canada, Transportation Tomorrow Survey and Strava.

Implementation, Operations and Maintenance Plans

- Review existing conditions and identify missing links.
- Identify an active transportation network including preferred facility types.
- Develop an implementation strategy including phasing, priorities and costing estimates.
- Establish educational, promotional and programming recommendations.
- Draft and finalize the plan.

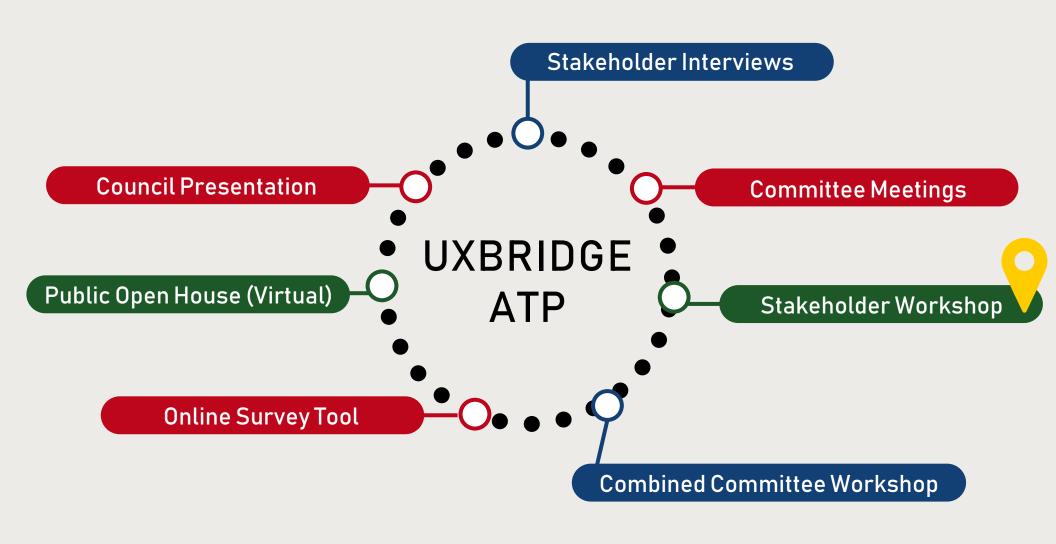








Engagement and Consultation











Shaping the Process

A number of activities have been undertaken to inform the study process and next steps:

6

stakeholder interviews

Interviews with:

Township staff Committees Interest groups 4

committee meetings

Meetings with:

Trails – August 6
AT – August 11
Accessibility – August 17
Combined Committee
meeting – September 22

1

online survey

140 participants

survey was active from July 24 to August 31

Plus calls / meetings with Township staff



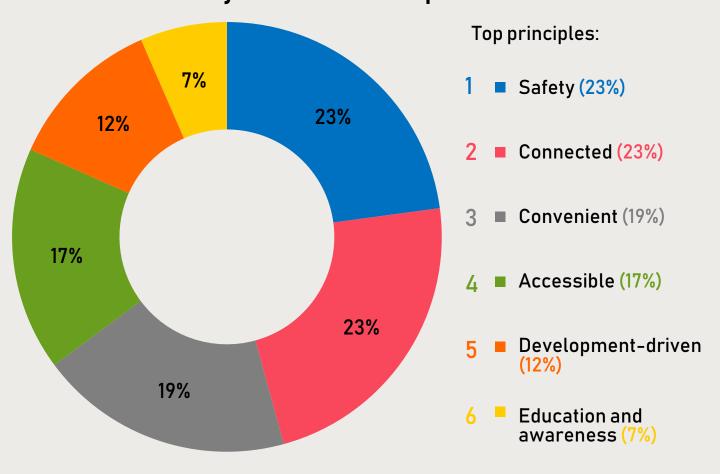






Online Survey Results

Most important principles that should be used to identify routes in the plan:



What does this tell us?

When identifying an active transportation network for Uxbridge, consideration should be given to routes that:

- Improve a user's sense of safety and ultimately makes them feel more comfortable to walk, bike and user other nonmotorized forms of travel.
- Connect to where people live and where they want to go,
- Are a convenient and realistic option for people to use for travel or recreation.









Online Survey Results 337 pins were placed a sample of the input: **Destinations:** Swimming pool, Farmers Market, Fields of Uxbridge, downtown area and shops, Second Wedge, Elgin Park, schools Barriers: Missing links to / from existing trails, lack of trail crossings, missing sidewalk connections, truck traffic, narrow roads **Bike Routes:** Wagg Road, Maple Bridge Trail, connections to Regional Forests, York-Durham Line, Ashworth Road Walking Routes: Elgin Park, to / from schools and shops, trails in Countryside Preserve Other Comments: Need improvement beyond downtown,









parking, trail conditions, crossings

Online Survey Results

Types of users:







22% **Cyclists**

What would encourage more people to be active:



22% a connected Township-wide network



19% more infrastructure / facilities such as paved shoulders



14% better connections to key destinations

Trip purpose:



39% For recreation and leisure



36% For health and fitness

Summary of priorities

- f I. Provide opportunities for people to walk, bike and be more active (27%)
- 2. Improve the quality of life and health of residents (20%)
- 3. Improve biking and walking as a viable transportation option (15%)









SWOT Analysis

Informed by input collected from stakeholder interviews and Committee meetings

Strengths	Weaknesses	Opportunities	Threats
 Presence of existing Committees 	Lack of driver awareness	Expand outreach through Committees	Lack of available funding programs
 Existing trail systems Engaged community volunteers and local groups Community is a good size for walking and biking Strong BIA Supportive Council and staff Common interest amongst residents to be more active 	 Sidewalk gaps Trails that do not make the final connection to a destination Previous developments have no / minimal provisions for AT infrastructure Lack of crosswalks Lack of funding to implement routes Lack of trailheads, parking and other 	 New policies to support AT and complete streets Uxbridge PS has a student AT Committee – leverage resources Shovel ready projects for the Federal COVID funding stream Work with partners, BIA and CoC to demonstrate economic benefits of investing in active transportation Create loops 	 Many stakeholders involved Population growth is slow – small tax base Constrained road right-of-way could limit desired facility type Temporary easements Aggregate industry is increasing, busy roads Maintenance costs Regional roads have many destinations
 Variety of trail levels that allow people to explore Uxbridge 	amenities at trailsNo safe way to get in / out of Uxbridge	connections • Cycle tourism benefits	along them - implementation determined by Region









Part 1 Outcomes

SW0T Analysis

Build a better understanding of the needs, interests and priorities to ensure recommendations are best-suited for success in Uxbridge.

Demand and Potential

Understand travel patterns (where people are walking and biking within the Township) and popular destinations, to help identify potential areas for improvement.

Existing Conditions

Identify and map where active transportation infrastructure currently exists within the Township including off-road trails and on-road routes on local and regional roads.

The findings from Part 1 will be used to inform the recommendations developed and identified in Part 2.









What's Next?

Future Directions, Strategies and Actions

- Undertake a strengths, weaknesses, opportunities and threats (SWOT) analysis of active transportation in Uxbridge.
- Assess demand and potential for active transportation in Uxbridge, using available data from Statistics Canada, Transportation Tomorrow Survey and Strava.

Implementation, Operations and Maintenance Plans

- Review existing conditions and identify missing links.
- Identify an active transportation network including preferred facility types.
- Develop an implementation strategy including phasing, priorities and costing estimates.
- Establish educational, promotional and programming recommendations.









Informing Part 2

A number of factors to be considered and integrated when undertaking Part 2:









Current standards and guidelines

Including the forth-coming update to OTM Book 18:

Cycling Facilities

Updates to the Durham Region RCP

Study is on-going: routes and recommendations will be integrated into the ATP

Planning and design principles

All ages and abilities
Complete streets
Equity

Input and feedback through on-going engagement









Applying OTM Book 18

Route Selection Criteria

Network connectivity

- Connectivity and physical barriers
- Directness
- Existing and potential future demand

Conflict mitigation

Social and economic factors

- Equity
- Social and economic trends
- Public and stakeholder input

Attractiveness

- Natural scenery and urban streetscape
- Tourism, business strategies and goals

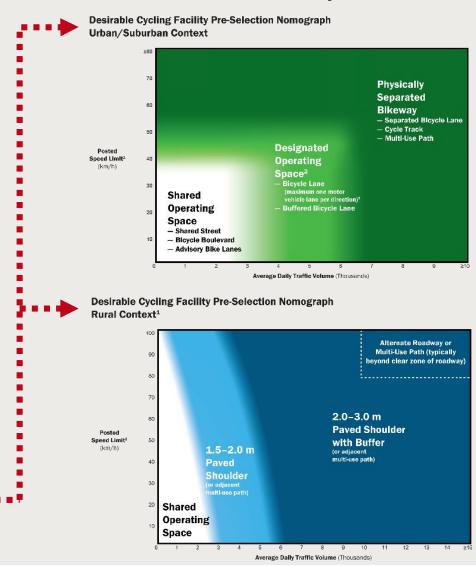
Feasibility

- Constructability
- Potential cost

How are the criteria applied?

 Potential new linkages (candidate routes) are assessed using the route selection criteria

Desirable Level of Separation











INTERACTIVE ACTIVIES





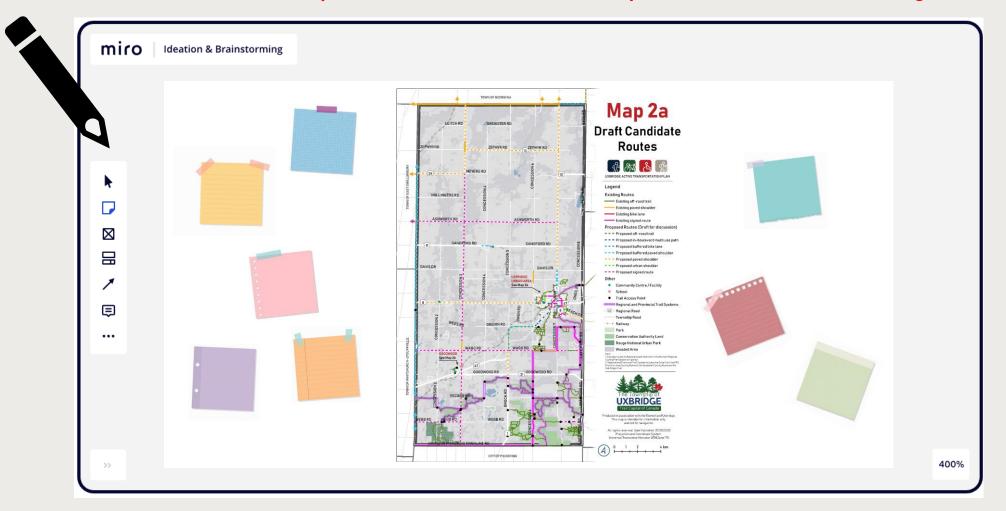






Draft Candidate Routes

The intent of this activity was to use an online interactive tool to collect your feedback and mark-ups on candidate active transportation routes in Uxbridge.



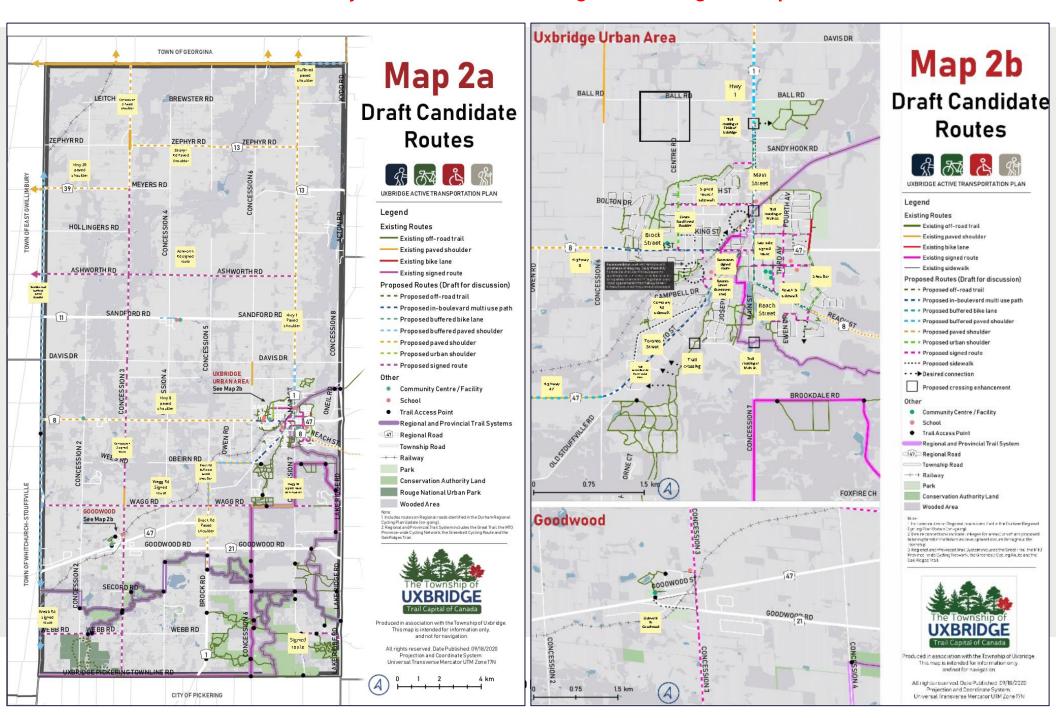








For reference, we have inserted a snippet of the input we received from the Town's AT, Trails and Accessibility Committees during a meeting on September 22, 2020.



We want to know:

- Additional candidate routes that should be explored / identified.
- Your comments and feedback on the proposed facility types and / or location of routes.
- Routes / projects that should be prioritized in the short term.

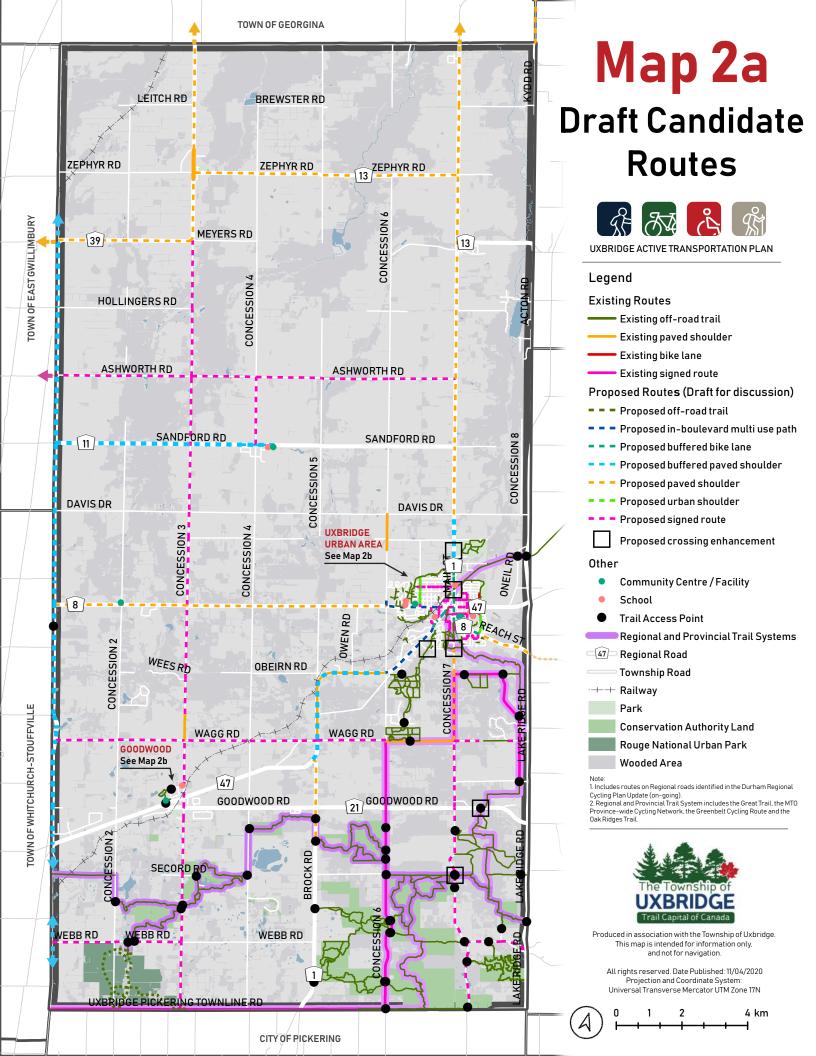
Please feel free to mark-up the following maps with your comments and input, and send this back to Lukas Gillham (lgillham@uxbridge.ca).

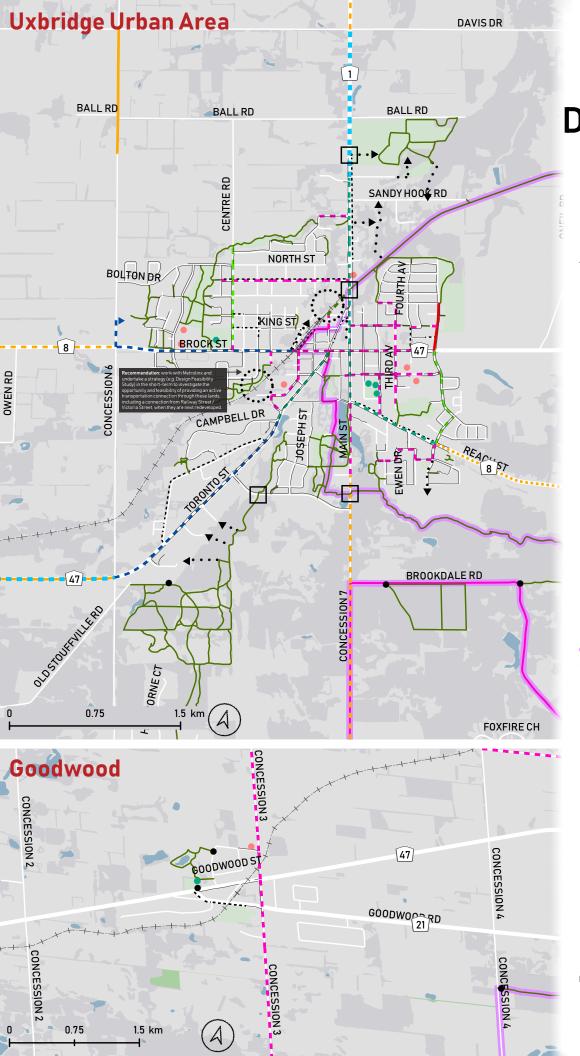












Map 2b

Draft Candidate Routes









UXBRIDGE ACTIVE TRANSPORTATION PLAN

Legend

Existing Routes

- Existing off-road trail
 - Existing paved shoulder
- Existing bike lane
- Existing signed route
- Existing sidewalk

Proposed Routes (Draft for discussion)

- Proposed off-road trail
- Proposed in-boulevard multi use path
- Proposed buffered bike lane
- Proposed buffered paved shoulder

Proposed urban shoulder

- Proposed paved shoulder
- Proposed signed route
- Proposed sidewalk
- Desired connection
- Proposed crossing enhancement

- Community Centre / Facility
- School
- Trail Access Point
- Regional and Provincial Trail System
- Regional Road
 - Township Road
- + Railway
- Park
- Conservation Authority Land
 - Wooded Area

- Note:
 Includes routes on Regional roads identified in the Durham Regional
 Cycling Plan Update (on-going).
 2. Desire connections indicate linkages (or areas) which are proposed
- 2. Desire Connections indicate timages (or all eas) which are proposed to be explored in the future as development occurs throughout the Township.

 3. Regional and Provincial Trail System includes the Great Trail, the MTO
- Province-wide Cycling Network, the Greenbelt Cycling Route and the



Produced in association with the Township of Uxbridge. This map is intended for information only, and not for navigation

All rights reserved. Date Published: 11/04/2020 Projection and Coordinate System: Universal Transverse Mercator UTM Zone 17N

Programming and Outreach

The intent of this activity was to use an online interactive tool to collect your feedback and input on potential programs and initiatives for Uxbridge.



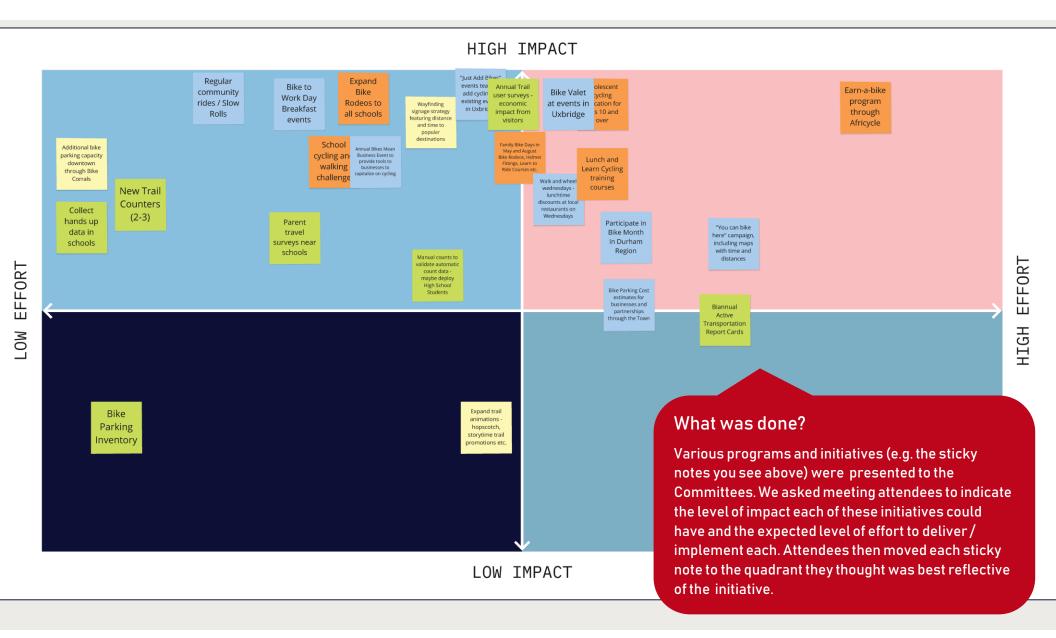








For reference, we have inserted a snippet of the input we received from the Town's AT, Trails and Accessibility Committees during a meeting on September 22, 2020.











We want to know:

Do you agree with the identified level of impact and effort for the proposed programs and initiatives?

Are there any other programs and / or initiatives that could be considered in the Township's AT Plan?

Please feel free to provide your comments and input to Lukas Gillham (lgillham@uxbridge.ca).











WRAP-UP











Next Steps

- 1. Summarize input received today.
- 2. Refine and prioritize routes to form part of a preferred active transportation network.
- 3. Refine and prioritize programming and outreach initiatives.
- 4. Prepare for upcoming consultation activities:
 - Final combined Committee Meeting (TBD)
 - Virtual Public Open House (TBD)
- 5. Prepare draft Active Transportation Plan (ATP) report.







Thank you for participating!

Visit the project website for more information and updates:

<u>www.uxbridge.ca/en/your-local-government/active-transportation-plan.aspx</u>

Contact Information

Lukas Gillham

Township of Uxbridge lgillham@uxbridge.ca 647-228-3916

Cristina Valente

WSP <u>cristina.valente@wsp.com</u> 647-730-7154









Appendix B
Existing Traffic Data

Brock St (R.R.08) @ Concession 6

TMC No: 1497 **Count ID: Count Date:** 0080800000 Intersection ID: 35702017157 AM Peak MD Peak Ped.→ 0.73 0.65 0.50 0.75 0.86 0.63 Ped.→ 08:15 0 12:15 0 4 % ↑ 0% 2% 20% 0 0 0 0 1 1 0 2 \uparrow_{\downarrow} Ped. Ped. 80 18 47 68 33 78 Trucks Trucks % PHF Trucks Trucks % PHF 0 0% 0.59 0% 0.71 5 182 112 0.88 14% 149 3% 0.74 0.33 0% 0 6% 0.91 0.79 10% 11 100 0 0% 0.76 0.89 3% 3 114 √ 78 1% 0.90 0.71 5% $\uparrow
\uparrow$ \uparrow \uparrow 0.59 0% 26 $\downarrow \leftarrow \uparrow \rightarrow 204$ PHF Trucks % Trucks Trucks % Trucks Cars 0 0 56 Ped. Ped. \uparrow_{\downarrow} Λ, 5 0 2020 **↓** % 3% 1% **↓** % % → Ped. → Ped. 0.64 0.51 0.89 0.91 0.72 PM Peak Total Count Ped.→ 0.54 0.72 0.75 16:45 0 7 hours* % % ↑ 5% 5% 5% 0 0 0 0 9 13 \uparrow_{\downarrow} Ped. Ped. 30 60 12 177 506 69 Trucks Trucks % PHF Cars Trucks Trucks % PHF 155 3% 0.71 45 1222 ← ↓ ↓ ↑ ↑ 1% 6 178 0.64 0% 0 18 4% 0.77 6% 4 ← 934 3% 32 0.95 3% 6 191 **√** 70 0 0% 0.83 4% 42 903 √ 819 2 0% 0 \uparrow \rightarrow 406 ↑ → 1819 0.80 3% \rightarrow \rightarrow PHF Trucks % Trucks Cars Trucks % Trucks 0 219 466 46 Ped. Ped. \uparrow_{\downarrow} \uparrow 3 2 2 1 9 23 **↓** 4% 2% 4% **↓** 4% 3% 1% → Ped. → Ped.

0.85 0.73 0.63

05/03/2018, Thu

TMC 15 Min Report

Brock St (R.R.08) @ Concession 6

TMC No: 05/03/2018, Thu Intersection ID: **Count ID: Count Date:** SOUTH APPROACH NORTH APPROACH EAST APPROACH **WEST APPROACH** Cars Heavies Ped Cars Heavies Ped Cars Heavies Ped Heavies Ped Trucks Trucks Trucks Cars Trucks Total Left Thru Thru Thru Right Thru Right Thru Thru Thru Thru Thru Right Thru Right Thru Right Thru Right Period 1 06:15 0 06:30 06:45 3 07:00 4 07:15 07:30 0 07:45 08:00 08:15 6 08:30 12 08:45 9 09:00 6 0* 0* 0* 0* 09:30 0* Period 2 11:45 4 12:00 3 12:15 12:30 6 12:45 4 13:00 4 13:15 6 Ω 13:30 9 Period 3 13:45 0* 0* 0* 0* 14:00 0* 0* 15:15 10 15:30 8 15:45 16:00 4 16:15 10 16:30 4 16:45 6 17:00 6 17:15 14 17:30 17:45 4 18:00 12

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TMC Tabular Report

BROCK ST W @ CENTRE RD

TMC No.: 0081200000 1425 Count ID: 34322013317 11/13/2013 Intersection ID: Count Date: 0.63 0.00 0.79 0.75 0.00 0.59 AM Peak: MD Peak: Ped. Ped. 8:15 am 11:45 am 13 16 0% 0% Trucks % Trucks % $\uparrow \downarrow$ 小 0 0 0 0 0 0 Ped. Ped. PHF PHF Cars Trucks Cars Trucks 55 0 88 24 19 37 0 0% 0.54 28 3% 0.81 19 383 5 187 168 5 3% 0.90 19 6% 295 0.84 24 26 0.63 0% 0.54 0% 0 0.00 0 0% 0.00 4% 0.91 144 0.72 7% 20 272 \rightarrow 327 20 168 6 0% 0% 0.00 0.00 0 PHF Trucks Cars PHF Trucks Cars 000 000 0 0 0 0 000 Ped. Ped. 0 0 0 Trucks % Trucks % $\uparrow \downarrow$ 小小 0% 0% 0% 0% Ped. Ped. → 0 0.00 0.00 0.00 0.00 0.00 0.00 0.68 0.00 0.61 PM Peak: **Total Count** Ped. Ped. \rightarrow 8 hours 4:45 pm 2% 0% 89 0% Trucks % Trucks % $\uparrow \downarrow$ Ŋ 404 0 0 0 Ped. Ped. 0 3 260 0 337 Cars Trucks Cars Trucks **PHF** 52 0 0% 0.88 234 2% 3 322 61 2072 300 3 1% 0.88 1735 60 3% 287 70 0.83 0% 0 0 0% 0.00 0% 0 4% 71 1598 3% 10 354 0.81 75 406 10 **→** 1858 **→** 0% 0 0.00 0% 0 0 Cars Trucks Trucks Cars PHF 000 0 0 0 4 Ped. 0 0 0 Ped. Trucks % Trucks % 0 0 0 0 小小 0 0% → Ped. → Ped. 0% 0% 0 0.00 00 .00 .0

Count Date: 11/13/2013

BROCK ST W @ CENTRE RD

							BROC	KSI	W @ CENT	KE KU						
	<u>NOI</u>	RTH APPROAC	<u>H</u>			EAST APPRO	<u>ACH</u>		<u>so</u>	JTH APPROAC	<u>H</u>		WES ¹	<u> FAPPROACH</u>		
Time	Cars	Trucks	Heavies	Ped	Cars	Trucks	Heavies	Ped	Cars	Trucks	Heavies	Ped	Cars	Trucks	Heavies Ped	Total
	Left Thru Righ	Left Thru Righ	t Left Thru Rig	ht	Left Thru Right	Left Thru Right	Left Thru Right	1	Left Thru Right	Left Thru Right	Left Thru Right		Left Thru Right	Left Thru Right	Left Thru Right	
Period 1																
6:15	1 0 13	0 0 0	0 0	0 0	0 22 0	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	1 8 0	0 0 0	0 0 0	0 45
6:30	3 0 12	0 0 1	0 0	0 0	0 21 0	0 0 0	0 0 0	3	0 0 0	0 0 0	0 0 0	0	1 2 0	0 0 0	0 0 0	0 43
6:45	5 0 15	0 0 0	0 0	0 1	0 34 4	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	2 7 0	0 1 0	0 0 0	0 69
7:00	3 0 24	0 0 0	0 0	0 0	0 30 3	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	2 16 0	0 1 0	0 0 0	0 79
7:15	3 0 16	0 0 0	0 0	0 0	0 52 1	0 1 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	2 10 0	0 1 0	0 0 0	0 86
7:30	1 0 20	0 0 0	0 0	0 2	0 68 1	0 2 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	0 22 0	0 0 0	0 0 0	0 116
7:45	5 0 17	0 0 0	0 0	0 0	0 53 3	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	3 18 0	0 2 0	0 0 0	0 101
8:00	3 0 17	1 0 0	0 0	0 0	0 50 4	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	6 35 0	0 3 0	0 0 0	0 119
8:15	12 0 26	0 0 0	0 0	0 5	0 68 4	0 3 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	4 48 0	0 4 0	0 0 0	0 174
8:30	12 0 28	0 0 0	0 0	0 5	0 82 6	0 5 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	5 97 0	0 4 0	0 0 0	0 244
8:45	6 0 19	0 0 0	0 0	0 1	0 90 9	0 3 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	4 64 0	0 5 0	0 0 0	0 201
9:00	22 0 14	0 0 0	0 0	0 2	0 47 6	0 7 1	0 0 0	0	0 0 0	0 0 0	0 0 0	0	12 56 0	0 7 0	0 0 0	0 174
Period 2																
11:45	7 0 8	0 0 0	0 0	0 3	0 46 17	0 2 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	7 40 0	0 1 0	0 0 0	0 131
12:00	4 0 3	0 0 0	0 0	0 4	0 41 6	0 2 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	3 38 0	0 2 0	0 0 0	0 103
12:15	5 0 3	0 0 0	0 0	0 1	0 48 4	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	9 30 0	1 1 0	0 0 0	0 102
12:30	8 0 5	0 0 0	0 0	0 8	0 33 10	0 1 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	5 36 0	0 2 0	0 0 0	0 108
12:45	4 0 6	1 0 0	0 0	0 1	0 28 10	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	6 33 0	0 3 0	0 0 0	92
13:00	11 0 4	0 0 0	0 0	0 2	0 37 8	0 4 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	5 46 0	0 0 0	0 0 0	0 117
13:15	6 0 5	0 0 0	0 0	0 0	0 37 4	0 1 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	2 36 0	0 2 0	0 0 0	93
13:30	2 0 2	0 0 0	0 0	0 1	0 42 7	0 2 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	3 41 0	0 2 0	0 0 0	0 102
Period 3																
15:15	9 0 10	0 0 0	0 0	0 8	0 71 14	0 7 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	6 50 0	0 1 0	0 0 0	1 177
15:30	17 0 10	0 0 0	0 0	0 23	0 56 4	0 6 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	16 62 0	0 4 0	0 0 0	1 199
15:45	5 0 4	0 0 0	0 0	0 7	0 51 9	0 7 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	25 70 0	0 9 0	0 0 0	0 187
16:00	10 0 11	0 0 0	0 0	0 7	0 63 12	0 0 1	0 0 0	0	0 0 0	0 0 0	0 0 0	0	12 73 0	0 1 0	0 0 0	0 190
16:15	14 0 5	1 0 0	0 0	0 2	0 65 13	0 2 1	0 0 0	0	0 0 0	0 0 0	0 0 0	0	11 86 0	0 2 0	0 0 0	1 203
16:30	10 0 8	0 0 0	0 0	0 0	0 63 4	0 1 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	13 82 0	0 2 0	0 0 0	0 183
16:45	19 0 5	0 0 0	0 0	0 0	0 79 16	0 1 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	15 111 0	0 1 0	0 0 0	0 247
17:00	14 0 2	0 0 0	0 0	0 4	0 67 10	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	21 90 0	0 0 0	0 0 0	0 208
17:15	8 0 9	0 0 0	0 0	0 0	0 85 14	0 1 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	19 72 0	0 6 0	0 0 0	0 214
17:30	11 0 6	0 0 0	0 0	0 0	0 69 16	0 1 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	15 81 0	0 3 0	0 0 0	0 202
17:45	10 0 6	1 0 0	0 0	0 0	0 70 7	0 1 1	0 0 0	1	0 0 0	0 0 0	0 0 0	0	33 87 0	0 0 0	0 0 0	0 217
18:00	10 0 4	0 0 0	0 0	0 2	0 67 8	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	19 51 0	0 1 0	0 0 0	0 162
18:00	0 0 0	0 0 2	0 0	0 0	0 0 1	0 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0	0 4 0	0 0 0	0 0 0	0 7
Print date:	8/18/2016	1	'	ı	1	I	I	1	l	I	I	ı	ı !		ı I	Page 2 of 2

Turning Movement Count Location Name: BOLTON DR / NORTH ST & CENTRE RD Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

										Turning	Move	ment Count (2 .	BOLTO	N DR /	NORTH	ST & C	ENTRE	RD)								
				N Approa	i ch RD					E Approac	ch T					S Approach	n D					W Approach	h R		Int. Total (15 min)	Int. Total (1 hr)
Start Time	Right N:W	Thru N:S	Left N:E	U-Turn N:N	Peds N:	Approach Total	Right E:N	Thru E:W	Left E:S	U-Turn E:E	Peds E:	Approach Total	Right S:E	Thru S:N	Left S:W	U-Turn S:S	Peds S:	Approach Total	Right W:S	Thru W:E	Left W:N	U-Turn W:W	Peds W:	Approach Total		
07:00:00	2	8	0	0	1	10	0	0	2	0	0	2	0	3	0	0	0	3	1	0	0	0	0	1	16	
07:15:00	0	5	0	0	4	5	0	0	1	0	0	1	1	2	0	0	0	3	0	0	1	0	2	1	10	
07:30:00	0	7	0	0	0	7	0	0	1	0	0	1	0	1	0	0	0	1	0	0	1	0	1	1	10	
07:45:00	1	6	0	0	0	7	1	0	3	0	0	4	0	2	0	0	0	2	4	3	0	0	0	7	20	56
08:00:00	1	9	0	0	0	10	1	1	2	0	0	4	0	5	1	0	0	6	3	2	0	0	2	5	25	65
08:15:00	3	9	0	0	0	12	1	1	2	0	0	4	0	1	2	0	0	3	4	2	2	0	0	8	27	82
08:30:00	8	11	0	0	4	19	0	3	1	0	1	4	1	2	2	0	0	5	5	2	2	0	5	9	37	109
08:45:00	0	4	0	0	0	4	0	2	2	0	0	4	2	6	3	0	0	11	16	9	7	0	2	32	51	140
09:00:00	0	4	0	0	1	4	1	2	0	0	0	3	1	4	1	0	0	6	2	2	1	0	0	5	18	133
09:15:00	3	9	0	0	1	12	2	1	2	0	0	5	1	8	0	0	1	9	3	0	0	0	1	3	29	135
09:30:00	2	6	0	0	0	8	0	1	0	0	0	1	0	5	0	0	0	5	3	1	0	0	1	4	18	116
09:45:00	0	9	1	0	1	10	0	0	2	0	0	2	2	4	1	0	0	7	2	4	0	0	5	6	25	90
***BREAK	***						-												-						-	
16:00:00	2	7	0	0	0	9	0	2	2	0	0	4	5	14	3	0	1	22	3	3	5	0	2	11	46	
16:15:00	2	8	0	0	3	10	2	4	1	0	0	7	3	6	3	0	0	12	2	3	1	0	1	6	35	
16:30:00	2	11	0	0	4	13	0	1	2	0	1	3	1	10	6	0	0	17	6	2	3	0	2	11	44	
16:45:00	3	8	1	0	4	12	2	4	3	0	1	9	4	12	6	0	0	22	5	4	5	0	0	14	57	182
17:00:00	3	5	0	0	4	8	1	2	1	0	0	4	1	12	7	0	0	20	2	5	0	0	4	7	39	175
17:15:00	5	9	3	0	4	17	2	2	1	0	0	5	5	12	3	0	2	20	4	4	3	0	4	11	53	193
17:30:00	1	5	2	0	0	8	3	2	0	0	2	5	2	5	5	0	1	12	2	2	3	0	0	7	32	181
17:45:00	1	7	1	1	0	10	1	4	0	0	1	5	4	8	6	0	1	18	2	2	4	0	0	8	41	165
18:00:00	2	9	0	0	1	11	1	2	0	0	0	3	2	4	4	0	1	10	2	4	3	0	1	9	33	159
18:15:00	0	9	1	0	1	10	0	1	0	0	0	1	0	6	6	0	2	12	1	2	0	0	0	3	26	132
18:30:00	2	4	0	0	1	6	1	2	2	0	2	5	4	6	3	0	1	13	4	0	1	0	1	5	29	129
18:45:00	4	9	0	0	0	13	0	0	0	0	0	0	2	5	3	0	0	10	4	4	2	0	1	10	33	121
Grand Total	47	178	9	1	34	235	19	37	30	0	8	86	41	143	65	0	10	249	80	60	44	0	35	184	754	-
Approach%	20%	75.7%	3.8%	0.4%		-	22.1%	43%	34.9%	0%		-	16.5%	57.4%	26.1%	0%		-	43.5%	32.6%	23.9%	0%		-		-
Totals %	6.2%	23.6%	1.2%	0.1%		31.2%	2.5%	4.9%	4%	0%		11.4%	5.4%	19%	8.6%	0%		33%	10.6%	8%	5.8%	0%		24.4%	-	-
Heavy	1	3	0	0		-	0	2	0	0		-	0	4	0	0		-	1	1	2	0		-	-	-
Heavy %	2.1%	1.7%	0%	0%		-	0%	5.4%	0%	0%		-	0%	2.8%	0%	0%		-	1.3%	1.7%	4.5%	0%		-	-	-
Bicycles	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-
Bicycle %	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-

Turning Movement Count Location Name: BOLTON DR / NORTH ST & CENTRE RD Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

								Peak	Hour:	08:00 A	M - 09:	00 AM Weat	her: So	attere	Cloud	ds (1.5 °	C)								
Start Time				N Appro	ach : RD					E Approach	h T					S Approa	ch RD					W Approac	c h DR		Int. Tot (15 mir
	Right	Thru	Left	U-Turn	Peds	Approach Total	Right	Thru	Left	U-Turn	Peds	Approach Total	Right	Thru	Left	U-Turn	Peds	Approach Total	Right	Thru	Left	U-Turn	Peds	Approach Total	
08:00:00	1	9	0	0	0	10	1	1	2	0	0	4	0	5	1	0	0	6	3	2	0	0	2	5	25
08:15:00	3	9	0	0	0	12	1	1	2	0	0	4	0	1	2	0	0	3	4	2	2	0	0	8	27
08:30:00	8	11	0	0	4	19	0	3	1	0	1	4	1	2	2	0	0	5	5	2	2	0	5	9	37
08:45:00	0	4	0	0	0	4	0	2	2	0	0	4	2	6	3	0	0	11	16	9	7	0	2	32	51
Grand Total	12	33	0	0	4	45	2	7	7	0	1	16	3	14	8	0	0	25	28	15	11	0	9	54	140
Approach%	26.7%	73.3%	0%	0%		-	12.5%	43.8%	43.8%	0%		-	12%	56%	32%	0%		-	51.9%	27.8%	20.4%	0%		-	-
Totals %	8.6%	23.6%	0%	0%		32.1%	1.4%	5%	5%	0%		11.4%	2.1%	10%	5.7%	0%		17.9%	20%	10.7%	7.9%	0%		38.6%	-
PHF	0.38	0.75	0	0		0.59	0.5	0.58	0.88	0		1	0.38	0.58	0.67	0		0.57	0.44	0.42	0.39	0		0.42	-
Heavy	1	3	0			4	0	1		0		1	0	0	0	0		0	0	1	2	0		3	
Heavy %	8.3%	9.1%	0%	0%		8.9%	0%	14.3%	0%	0%		6.3%	0%	0%	0%	0%		0%	0%	6.7%	18.2%	0%		5.6%	-
Lights	11	30	0	0		41	2	6	7	0		15	3	14	8	0		25	28	14	9	0		51	
Lights %	91.7%	90.9%	0%	0%		91.1%	100%	85.7%	100%	0%		93.8%	100%	100%	100%	0%		100%	100%	93.3%	81.8%	0%		94.4%	-
Single-Unit Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Buses	1	3	0	0		4	0	1	0	0		1	0	0	0	0		0	0	1	2	0		3	-
Buses %	8.3%	9.1%	0%	0%		8.9%	0%	14.3%	0%	0%		6.3%	0%	0%	0%	0%		0%	0%	6.7%	18.2%	0%		5.6%	-
Articulated Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Bicycles on Road	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Bicycles on Road %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	-	4	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	8	-	-
Pedestrians%	-	-	-	-	28.6%		-	-	-	-	7.1%		-	-	-	-	0%		-	-	-	-	57.1%		-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-
icycles on Crosswalk%	-	-	-	-	0%		-	-	-	-	0%		-	-	-	-	0%		-	-	-	-	7.1%		-

Turning Movement Count Location Name: BOLTON DR / NORTH ST & CENTRE RD Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

								Pe	ak Hou	ır: 04:30	PM - (05:30 PM We	ather: F	ew Clo	uds (10	.39 °C)									
Start Time				N Approa	ch RD					E Approacl	n T					S Approach	h D					W Approac	h R		Int. Tota (15 min)
	Right	Thru	Left	U-Turn	Peds	Approach Total	Right	Thru	Left	U-Turn	Peds	Approach Total	Right	Thru	Left	U-Turn	Peds	Approach Total	Right	Thru	Left	U-Turn	Peds	Approach Total	
16:30:00	2	11	0	0	4	13	0	1	2	0	1	3	1	10	6	0	0	17	6	2	3	0	2	11	44
16:45:00	3	8	1	0	4	12	2	4	3	0	1	9	4	12	6	0	0	22	5	4	5	0	0	14	57
17:00:00	3	5	0	0	4	8	1	2	1	0	0	4	1	12	7	0	0	20	2	5	0	0	4	7	39
17:15:00	5	9	3	0	4	17	2	2	1	0	0	5	5	12	3	0	2	20	4	4	3	0	4	11	53
Grand Total	13	33	4	0	16	50	5	9	7	0	2	21	11	46	22	0	2	79	17	15	11	0	10	43	193
Approach%	26%	66%	8%	0%		-	23.8%	42.9%	33.3%	0%		-	13.9%	58.2%	27.8%	0%		-	39.5%	34.9%	25.6%	0%		-	-
Totals %	6.7%	17.1%	2.1%	0%		25.9%	2.6%	4.7%	3.6%	0%		10.9%	5.7%	23.8%	11.4%	0%		40.9%	8.8%	7.8%	5.7%	0%		22.3%	-
PHF	0.65	0.75	0.33	0		0.74	0.63	0.56	0.58	0		0.58	0.55	0.96	0.79	0		0.9	0.71	0.75	0.55	0		0.77	-
Heavy		0	0	0		0	0	0	0	0		0	0	1	0	0		1	0	0	0	0		0	
Heavy %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	2.2%	0%	0%		1.3%	0%	0%	0%	0%		0%	-
Lights	13	29	4	0		46	5	9	7	0		21	11	45	21	0		77	17	15	11	0		43	
Lights %	100%	87.9%	100%	0%		92%	100%	100%	100%	0%		100%	100%	97.8%	95.5%	0%		97.5%	100%	100%	100%	0%		100%	-
Single-Unit Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Buses	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	-
Buses %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	-
Articulated Trucks	0	0	0	0		0	0	0	0	0		0	0	1	0	0		1	0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%	0%		0%	0%	0%	0%	0%		0%	0%	2.2%	0%	0%		1.3%	0%	0%	0%	0%		0%	-
Bicycles on Road	0	4	0	0		4	0	0	0	0		0	0	0	1	0		1	0	0	0	0		0	-
Bicycles on Road %	0%	12.1%	0%	0%		8%	0%	0%	0%	0%		0%	0%	0%	4.5%	0%		1.3%	0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	-	16	-	-	-	-	-	2	-	-	-	-	-	2	-	-	-	-	-	10	-	-
Pedestrians%	-	-	-	-	53.3%		-	-	-	-	6.7%		-	-	-	-	6.7%		-	-	-	-	33.3%		-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	-	0%		-	-	-	-	0%		-	-	-	-	0%		-	-	-	-	0%		-



Peak Hour: 08:00 AM - 09:00 AM Weather: Scattered Clouds (1.5 °C)



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(D) mapbox

Peak Hour: 04:30 PM - 05:30 PM Weather: Few Clouds (10.39 °C) Legend: ### (#.# %) [#.##] TOTAL VEHICLES (HEAVY %) [PHF] Dr CarmodyLn Bicycles on Crosswalk Pedestrians 0 2 0 2 10 0

Bicycle %

Turning Movement Count Location Name: BOLTON DR & CONCESSION RD 6 Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					''	g w	JVCIIICIII	Count (1	. DOLI	ON DR & CONCE		0,					
Start Time			N App CONCESS		3				proach TON DR				S App CONCES	oroach SION RD (6	Int. Total (15 min)	Int. To (1 hr
Start Time	Thru N:S	Left N:E	U-Turn N:N	Peds N:	Approach Total	Right E:N	Left E:S	U-Turn E:E	Peds E:	Approach Total	Right S:E	Thru S:N	U-Turn S:S	Peds S:	Approach Total		
07:00:00	19	0	0	0	19	2	4	0	0	6	0	10	0	0	10	35	
07:15:00	21	0	0	0	21	2	8	0	0	10	3	11	0	0	14	45	
07:30:00	21	5	0	0	26	1	3	0	0	4	1	8	0	0	9	39	
07:45:00	39	2	0	0	41	1	3	0	0	4	0	6	0	0	6	51	170
08:00:00	32	0	0	0	32	0	3	0	0	3	4	5	0	0	9	44	179
08:15:00	21	0	0	0	21	0	5	0	0	5	4	17	0	0	21	47	181
08:30:00	22	2	0	0	24	1	9	0	1	10	1	8	0	0	9	43	185
08:45:00	23	0	0	0	23	3	19	0	0	22	5	12	0	0	17	62	196
09:00:00	19	1	0	0	20	3	4	0	0	7	4	15	0	0	19	46	198
09:15:00	20	1	0	0	21	3	6	0	0	9	3	15	0	0	18	48	199
09:30:00	27	1	0	0	28	0	3	0	0	3	4	17	0	0	21	52	208
09:45:00	21	1	0	0	22	1	9	0	0	10	3	15	0	0	18	50	196
***BREAK	***															1	
16:00:00	21	1	0	0	22	2	4	0	2	6	5	36	0	0	41	69	
16:15:00	21	3	0	0	24	0	3	1	0	4	9	32	0	0	41	69	
16:30:00	21	2	0	0	23	1	6	0	0	7	7	32	0	0	39	69	
16:45:00	20	3	0	0	23	2	8	0	0	10	18	33	0	0	51	84	291
17:00:00	19	0	0	0	19	3	8	0	0	11	13	43	0	1	56	86	308
17:15:00	19	2	0	0	21	0	10	0	0	10	9	34	0	0	43	74	313
17:30:00	9	2	0	0	11	1	7	0	0	8	12	31	0	2	43	62	306
17:45:00	20	1	0	0	21	3	1	0	0	4	13	34	0	0	47	72	294
18:00:00	16	1	0	0	17	3	10	1	0	14	13	18	0	0	31	62	270
18:15:00	10	4	0	0	14	1	6	0	0	7	16	18	0	0	34	55	251
18:30:00	9	1	0	0	10	0	3	0	0	3	6	16	0	0	22	35	224
18:45:00	12	0	0	0	12	2	5	0	0	7	3	15	0	0	18	37	189
Grand Total	482	33	0	0	515	35	147	2	3	184	156	481	0	3	637	1336	-
Approach%	93.6%	6.4%	0%		-	19%	79.9%	1.1%		-	24.5%	75.5%	0%		-	-	-
Totals %	36.1%	2.5%	0%		38.5%	2.6%	11%	0.1%		13.8%	11.7%	36%	0%		47.7%	-	-
Heavy	22	5	0		-	1	3	0		-	2	12	0		-	-	-
Heavy %	4.6%	15.2%	0%		-	2.9%	2%	0%		-	1.3%	2.5%	0%		-	-	-
Bicycles	-	-	-		-	-	-	-		-	-	-	-		-	-	-

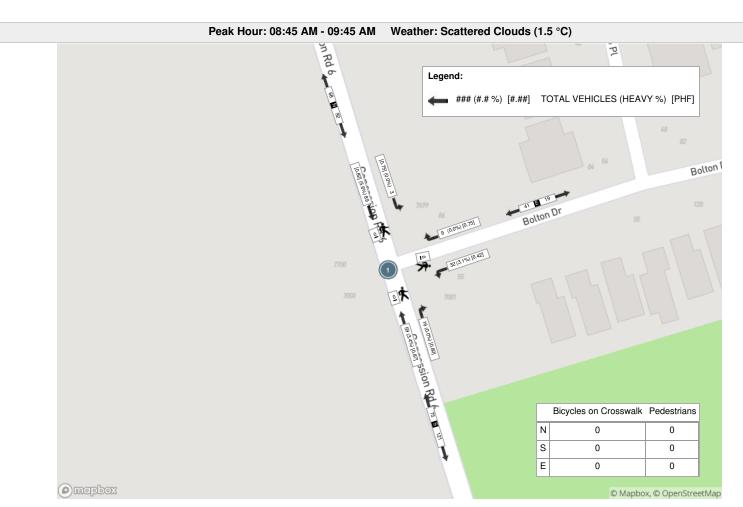
Turning Movement Count Location Name: BOLTON DR & CONCESSION RD 6 Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					Peak Hour: 08:45	5 AM - 09	:45 AM	Weathe	r: Scatte	ered Clouds (1.5 °	C)					
Start Time			N App CONCES	oroach SION RD	6				oroach ON DR				S App CONCES	roach SION RD	6	Int. Total (15 min)
	Thru	Left	U-Turn	Peds	Approach Total	Right	Left	U-Turn	Peds	Approach Total	Right	Thru	U-Turn	Peds	Approach Total	
08:45:00	23	0	0	0	23	3	19	0	0	22	5	12	0	0	17	62
09:00:00	19	1	0	0	20	3	4	0	0	7	4	15	0	0	19	46
09:15:00	20	1	0	0	21	3	6	0	0	9	3	15	0	0	18	48
09:30:00	27	1	0	0	28	0	3	0	0	3	4	17	0	0	21	52
Grand Total	89	3	0	0	92	9	32	0	0	41	16	59	0	0	75	208
Approach%	96.7%	3.3%	0%		-	22%	78%	0%		-	21.3%	78.7%	0%		-	-
Totals %	42.8%	1.4%	0%		44.2%	4.3%	15.4%	0%		19.7%	7.7%	28.4%	0%		36.1%	-
PHF	0.82	0.75	0		0.82	0.75	0.42	0		0.47	0.8	0.87	0		0.89	-
Heavy	5	0	0		5	0	1	0		1	0	2	0		2	
Heavy %	5.6%	0%	0%		5.4%	0%	3.1%	0%		2.4%	0%	3.4%	0%		2.7%	-
Lights	84	3	0		87	9	31	0		40	16	57	0		73	
Lights %	94.4%	100%	0%		94.6%	100%	96.9%	0%		97.6%	100%	96.6%	0%		97.3%	-
Single-Unit Trucks	4	0	0		4	0	0	0		0	0	0	0		0	-
Single-Unit Trucks %	4.5%	0%	0%		4.3%	0%	0%	0%		0%	0%	0%	0%		0%	-
Buses	0	0	0		0	0	1	0		1	0	1	0		1	-
Buses %	0%	0%	0%		0%	0%	3.1%	0%		2.4%	0%	1.7%	0%		1.3%	-
Articulated Trucks	1	0	0		1	0	0	0		0	0	1	0		1	-
Articulated Trucks %	1.1%	0%	0%		1.1%	0%	0%	0%		0%	0%	1.7%	0%		1.3%	-
Bicycles on Road	0	0	0		0	0	0	0		0	0	0	0		0	-
Bicycles on Road %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Pedestrians%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-

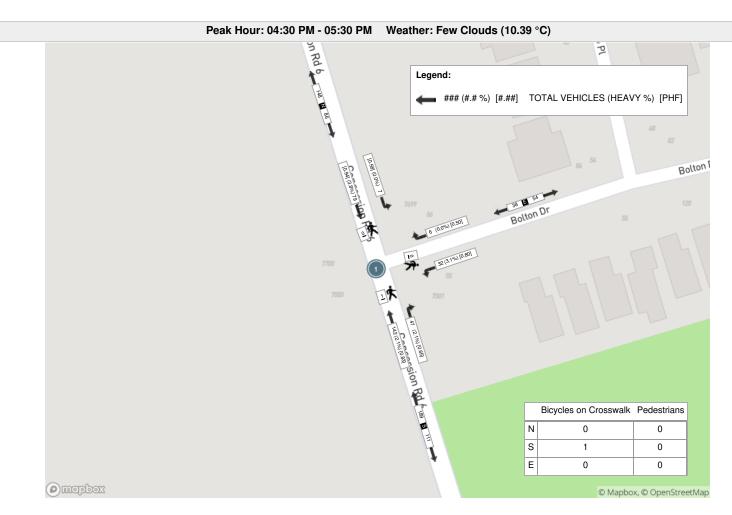
Turning Movement Count Location Name: BOLTON DR & CONCESSION RD 6 Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					Peak Hour: 04:	30 PM - (05:30 PM	Weath	er: Few	Clouds (10.39 °C)						
Start Time				proach SION RD (6			E App BOLT						oroach SION RD 6	3	Int. Total (15 min)
	Thru	Left	U-Turn	Peds	Approach Total	Right	Left	U-Turn	Peds	Approach Total	Right	Thru	U-Turn	Peds	Approach Total	
16:30:00	21	2	0	0	23	1	6	0	0	7	7	32	0	0	39	69
16:45:00	20	3	0	0	23	2	8	0	0	10	18	33	0	0	51	84
17:00:00	19	0	0	0	19	3	8	0	0	11	13	43	0	1	56	86
17:15:00	19	2	0	0	21	0	10	0	0	10	9	34	0	0	43	74
Grand Total	79	7	0	0	86	6	32	0	0	38	47	142	0	1	189	313
Approach%	91.9%	8.1%	0%		-	15.8%	84.2%	0%		-	24.9%	75.1%	0%		-	-
Totals %	25.2%	2.2%	0%		27.5%	1.9%	10.2%	0%		12.1%	15%	45.4%	0%		60.4%	-
PHF	0.94	0.58	0		0.93	0.5	0.8	0		0.86	0.65	0.83	0		0.84	-
Heavy	3	0	0		3	0	1	0		1	1	3	0		4	
Heavy %	3.8%	0%	0%		3.5%	0%	3.1%	0%		2.6%	2.1%	2.1%	0%		2.1%	-
Lights	76	7	0		83	6	31	0		37	46	138	0		184	
Lights %	96.2%	100%	0%		96.5%	100%	96.9%	0%		97.4%	97.9%	97.2%	0%		97.4%	-
Single-Unit Trucks	2	0	0		2	0	1	0		1	1	2	0		3	-
Single-Unit Trucks %	2.5%	0%	0%		2.3%	0%	3.1%	0%		2.6%	2.1%	1.4%	0%		1.6%	-
Buses	1	0	0		1	0	0	0		0	0	0	0		0	-
Buses %	1.3%	0%	0%		1.2%	0%	0%	0%		0%	0%	0%	0%		0%	-
Articulated Trucks	0	0	0		0	0	0	0		0	0	1	0		1	-
Articulated Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0.7%	0%		0.5%	-
Bicycles on Road	0	0	0		0	0	0	0		0	0	1	0		1	-
Bicycles on Road %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0.7%	0%		0.5%	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Pedestrians%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	1	-	-
Bicycles on Crosswalk%	-	-	-	0%		-	-	-	0%		-	-	-	100%		-









Turning Movement Count (3 . BOLTON DR & QUAKER VILLAGE DR) E Approach S Approach W Approach Int. Total Int. Total BOLTON DR QUAKER VILLAGE DR BOLTON DR (15 min) (1 hr) Start Time U-Turn Right U-Turn Right Thru U-Turn Thru Left Peds Left Peds Peds Approach Total Approach Total Approach Total E:S E:E S:S S: W:E W:W E:W E: S:E S:W W:S W: 07:00:00 07:15:00 07:30:00 07:45:00 08:00:00 08:15:00 08:30:00 08:45:00 09:00:00 09:15:00 09:30:00 09:45:00 ***BREAK***

16:00:00	2	4	0	0	Ь	'	2	0	2	9	0	3	0	0	3	18	
16:15:00	4	3	0	0	7	2	0	0	0	2	3	7	1	0	11	20	
16:30:00	5	4	0	2	9	7	6	0	2	13	2	6	0	0	8	30	
16:45:00	7	6	0	2	13	5	4	0	0	9	1	8	0	0	9	31	99
17:00:00	9	6	0	0	15	3	1	0	1	4	2	5	0	0	7	26	107
17:15:00	3	3	0	0	6	5	3	0	3	8	3	4	0	0	7	21	108
17:30:00	2	3	1	0	6	2	2	0	0	4	0	3	0	0	3	13	91
17:45:00	10	0	0	0	10	3	2	0	1	5	2	9	0	0	11	26	86
18:00:00	7	2	0	1	9	2	0	0	0	2	1	5	0	0	6	17	77
18:15:00	5	2	0	0	7	1	0	0	2	1	2	5	0	0	7	15	71
18:30:00	5	1	0	0	6	2	1	0	3	3	0	3	0	0	3	12	70
18:45:00	3	6	0	0	9	4	3	0	4	7	1	6	0	0	7	23	67
Grand Total	83	66	1	10	150	90	49	1	39	140	31	100	1	2	132	422	-
Approach%	55.3%	44%	0.7%		-	64.3%	35%	0.7%		-	23.5%	75.8%	0.8%		-	-	-
Totals %	19.7%	15.6%	0.2%		35.5%	21.3%	11.6%	0.2%		33.2%	7.3%	23.7%	0.2%		31.3%	-	-
Heavy	0	2	0		-	2	2	0		-	1	3	0		-	-	-
Heavy %	0%	3%	0%		-	2.2%	4.1%	0%		-	3.2%	3%	0%		-	-	-
Bicycles	-	-	-		-	-	-	-		-	-	-	-		-	-	-
Bicycle %	-	-	-		-	-	-	-		-	-	-	-		-	-	-

Turning Movement Count Location Name: BOLTON DR & QUAKER VILLAGE DR Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

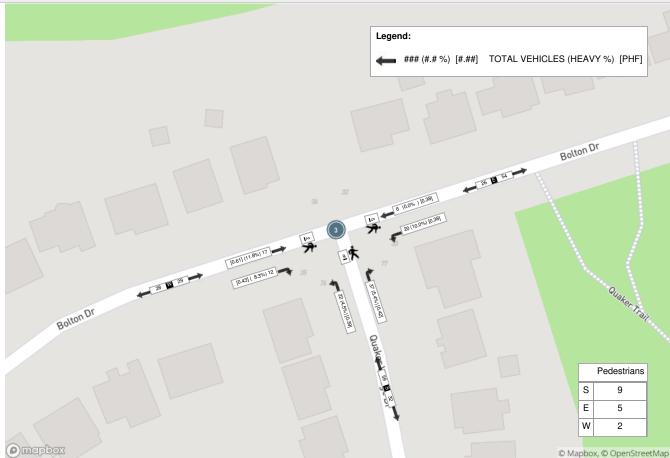
					Peak Hour: 08	- MA 00:	09:00 AN	l Weath	her: Scatt	tered Clouds (1.5	°C)					
Start Time				proach TON DR					proach VILLAGE DI	R				proach TON DR		Int. Tot (15 mi
	Thru	Left	U-Turn	Peds	Approach Total	Right	Left	U-Turn	Peds	Approach Total	Right	Thru	U-Turn	Peds	Approach Total	
08:00:00	0	2	0	0	2	3	2	0	4	5	2	2	0	1	4	11
08:15:00	1	4	0	4	5	2	0	0	3	2	2	5	0	1	7	14
08:30:00	4	13	0	0	17	10	6	0	0	16	7	3	0	0	10	43
08:45:00	1	1	0	1	2	22	14	0	2	36	1	7	0	0	8	46
Grand Total	6	20	0	5	26	37	22	0	9	59	12	17	0	2	29	114
Approach%	23.1%	76.9%	0%		-	62.7%	37.3%	0%		-	41.4%	58.6%	0%		-	-
Totals %	5.3%	17.5%	0%		22.8%	32.5%	19.3%	0%		51.8%	10.5%	14.9%	0%		25.4%	-
PHF	0.38	0.38	0		0.38	0.42	0.39	0		0.41	0.43	0.61	0		0.73	-
Heavy	0	2	0		2	2	1	0		3	1	2	0		3	<u>-</u>
Heavy %	0%	10%	0%		7.7%	5.4%	4.5%	0%		5.1%	8.3%	11.8%	0%		10.3%	-
Lights	6	18	0		24	35	21	0		56	11	15	0		26	<u>-</u>
Lights %	100%	90%	0%		92.3%	94.6%	95.5%	0%		94.9%	91.7%	88.2%	0%		89.7%	-
Single-Unit Trucks	0	0	0		0	0	0	0		0	0	1	0		1	-
ingle-Unit Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	5.9%	0%		3.4%	-
Buses	0	2	0		2	2	1	0		3	1	1	0		2	-
Buses %	0%	10%	0%		7.7%	5.4%	4.5%	0%		5.1%	8.3%	5.9%	0%		6.9%	-
Bicycles on Road	0	0	0		0	0	0	0		0	0	0	0		0	-
Bicycles on Road %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	5	-	-	-	-	9	-	-	-	-	2	-	-
Pedestrians%	-	-	-	31.3%		-	-	-	56.3%		-	-	-	12.5%		-

Turning Movement Count Location Name: BOLTON DR & QUAKER VILLAGE DR Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					Peak Hour: 0	4:30 PM -	· 05:30 PI	vi Weat	ner: Fev	v Clouds (10.39 °C	<i>i</i>)					
Start Time				oroach ON DR				S App QUAKER V		R				oroach ON DR		Int. Tota (15 min
	Thru	Left	U-Turn	Peds	Approach Total	Right	Left	U-Turn	Peds	Approach Total	Right	Thru	U-Turn	Peds	Approach Total	
16:30:00	5	4	0	2	9	7	6	0	2	13	2	6	0	0	8	30
16:45:00	7	6	0	2	13	5	4	0	0	9	1	8	0	0	9	31
17:00:00	9	6	0	0	15	3	1	0	1	4	2	5	0	0	7	26
17:15:00	3	3	0	0	6	5	3	0	3	8	3	4	0	0	7	21
Grand Total	24	19	0	4	43	20	14	0	6	34	8	23	0	0	31	108
Approach%	55.8%	44.2%	0%		-	58.8%	41.2%	0%		-	25.8%	74.2%	0%		-	-
Totals %	22.2%	17.6%	0%		39.8%	18.5%	13%	0%		31.5%	7.4%	21.3%	0%		28.7%	-
PHF	0.67	0.79	0		0.72	0.71	0.58	0		0.65	0.67	0.72	0		0.86	-
Heavy	0	0	0		0	0	0	0		0	0	0	0		0	
Heavy %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Lights	23	19	0		42	20	12	0		32	8	23	0		31	
Lights %	95.8%	100%	0%		97.7%	100%	85.7%	0%		94.1%	100%	100%	0%		100%	-
Single-Unit Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Single-Unit Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Buses	0	0	0		0	0	0	0		0	0	0	0		0	-
Buses %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Bicycles on Road	1	0	0		1	0	2	0		2	0	0	0		0	-
Bicycles on Road %	4.2%	0%	0%		2.3%	0%	14.3%	0%		5.9%	0%	0%	0%		0%	-
Pedestrians	-	-	-	4	-	-	-	-	6	-	-	-	-	0	-	-
Pedestrians%	-	-	-	40%		-	-	-	60%		-	-	-	0%		-

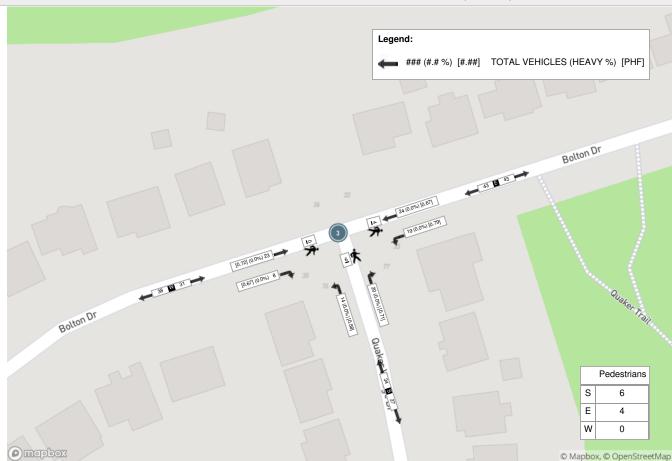


Peak Hour: 08:00 AM - 09:00 AM Weather: Scattered Clouds (1.5 °C)





Peak Hour: 04:30 PM - 05:30 PM Weather: Few Clouds (10.39 °C)



Bicycle %

Turning Movement Count Location Name: CENTRE RD & OAKSIDE DR Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

						Turning	g Movem	ent Cour	it (4 . CE	ENTRE RD & OAKS	SIDE DR)						
Start Time			N Ap	proach TRE RD				E App OAKS	roach IDE DR				S App CENT	roach RE RD		Int. Total (15 min)	Int. Tota (1 hr)
Start Time	Thru N:S	Left N:E	U-Turn N:N	Peds N:	Approach Total	Right E:N	Left E:S	U-Turn E:E	Peds E:	Approach Total	Right S:E	Thru S:N	U-Turn S:S	Peds S:	Approach Total		
07:00:00	4	0	0	0	4	1	7	0	1	8	0	3	0	0	3	15	
07:15:00	0	0	0	0	0	1	5	0	4	6	1	2	0	0	3	9	
07:30:00	1	0	0	0	1	0	4	0	4	4	1	1	0	0	2	7	
07:45:00	3	0	0	0	3	0	5	0	4	5	0	3	0	0	3	11	42
08:00:00	7	0	0	0	7	0	2	0	3	2	0	6	0	0	6	15	42
08:15:00	6	0	0	0	6	1	8	0	0	9	2	2	0	0	4	19	52
08:30:00	5	1	0	0	6	0	12	0	4	12	4	2	0	0	6	24	69
08:45:00	1	0	0	0	1	1	3	0	3	4	7	4	0	0	11	16	74
09:00:00	2	0	0	0	2	0	3	0	1	3	3	3	0	0	6	11	70
09:15:00	4	1	0	0	5	0	8	0	1	8	7	4	0	0	11	24	75
09:30:00	3	0	0	0	3	0	8	0	2	8	3	1	0	0	4	15	66
09:45:00	0	1	0	0	1	0	8	0	3	8	1	3	0	0	4	13	63
BREA	(1		ı									1 1	1		1
16:00:00	3	1	0	0	4	0	6	0	2	6	8	9	0	0	17	27	
16:15:00	5	1	0	0	6	1	6	0	3	7	5	5	0	0	10	23	
16:30:00	7	1	0	0	8	0	6	0	5	6	6	7	0	0	13	27	
16:45:00	6	1	0	0	7	1	5	0	5	6	9	10	0	0	19	32	109
17:00:00	6	1	0	0	7	0	4	0	2	4	9	4	0	0	13	24	106
17:15:00	8	0	0	0	8	1	7	0	7	8	7	10	0	0	17	33	116
17:30:00	4	1	0	0	5	0	4	0	2	4	7	5	0	0	12	21	110
17:45:00	6	1	0	0	7	0	4	0	1	4	7	5	0	0	12	23	101
18:00:00	6	0	0	0	6	0	7	0	1	7	3	5	0	0	8	21	98
18:15:00	4	0	0	0	4	0	5	0	3	5	5	1	0	0	6	15	80
18:30:00	2	0	0	0	2	0	2	0	1	2	6	2	0	0	8	12	71
18:45:00	7	0	0	0	7	1	7	0	0	8	4	3	0	0	7	22	70
Grand Total	100	10	0	0	110	8	136	0	62	144	105	100	0	0	205	459	-
Approach%	90.9%	9.1%	0%		-	5.6%	94.4%	0%		-	51.2%	48.8%	0%		-		-
Totals %	21.8%	2.2%	0%		24%	1.7%	29.6%	0%		31.4%	22.9%	21.8%	0%		44.7%	-	-
Heavy	2	0	0		-	1	2	0		-	3	3	0		-	-	-
Heavy %	2%	0%	0%		-	12.5%	1.5%	0%		-	2.9%	3%	0%		-	-	-
Bicycles	-	-	-		-	-	-	-		-	-	-	-		-	-	-

Turning Movement Count Location Name: CENTRE RD & OAKSIDE DR Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					Peak Hour: 08:30	AM - 09	:30 AM	Weathe	r: Scatte	ered Clouds (1.5 °C	C)					
Start Time			N App CENTI						proach SIDE DR				S App CENT			Int. Total (15 min)
	Thru	Left	U-Turn	Peds	Approach Total	Right	Left	U-Turn	Peds	Approach Total	Right	Thru	U-Turn	Peds	Approach Total	
08:30:00	5	1	0	0	6	0	12	0	4	12	4	2	0	0	6	24
08:45:00	1	0	0	0	1	1	3	0	3	4	7	4	0	0	11	16
09:00:00	2	0	0	0	2	0	3	0	1	3	3	3	0	0	6	11
09:15:00	4	1	0	0	5	0	8	0	1	8	7	4	0	0	11	24
Grand Total	12	2	0	0	14	1	26	0	9	27	21	13	0	0	34	75
Approach%	85.7%	14.3%	0%		-	3.7%	96.3%	0%		-	61.8%	38.2%	0%		-	-
Totals %	16%	2.7%	0%		18.7%	1.3%	34.7%	0%		36%	28%	17.3%	0%		45.3%	-
PHF	0.6	0.5	0		0.58	0.25	0.54	0		0.56	0.75	0.81	0		0.77	-
Heavy	2	0	0		2	0	1	0		1	2	1	0		3	
Heavy %	16.7%	0%	0%		14.3%	0%	3.8%	0%		3.7%	9.5%	7.7%	0%		8.8%	-
Lights	10	2	0		12	1	25	0		26	19	12	0		31	·
Lights %	83.3%	100%	0%		85.7%	100%	96.2%	0%		96.3%	90.5%	92.3%	0%		91.2%	-
Single-Unit Trucks	0	0	0		0	0	0	0		0	1	0	0		1	-
Single-Unit Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	4.8%	0%	0%		2.9%	-
Buses	2	0	0		2	0	1	0		1	1	1	0		2	-
Buses %	16.7%	0%	0%		14.3%	0%	3.8%	0%		3.7%	4.8%	7.7%	0%		5.9%	-
Bicycles on Road	0	0	0		0	0	0	0		0	0	0	0		0	-
Bicycles on Road %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	0	-	-	-	-	9	-	-	-	-	0	-	-
Pedestrians%	-	-	-	0%		-	-	-	100%		-	-	-	0%		-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-

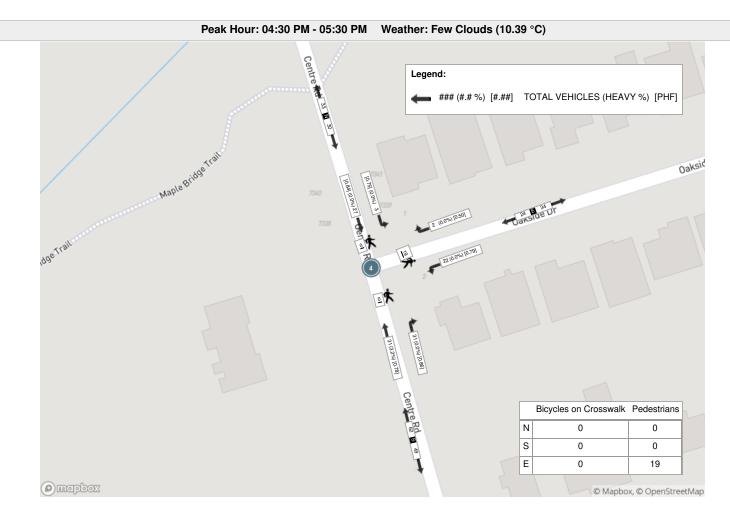
Turning Movement Count Location Name: CENTRE RD & OAKSIDE DR Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					Peak Hour: 04:	30 PM -	05:30 PN	l Weath	er: Few	Clouds (10.39 °C)	1					
Start Time			N App CENT	roach RE RD					roach IDE DR				S App CENT	roach RE RD		Int. Total (15 min)
	Thru	Left	U-Turn	Peds	Approach Total	Right	Left	U-Turn	Peds	Approach Total	Right	Thru	U-Turn	Peds	Approach Total	
16:30:00	7	1	0	0	8	0	6	0	5	6	6	7	0	0	13	27
16:45:00	6	1	0	0	7	1	5	0	5	6	9	10	0	0	19	32
17:00:00	6	1	0	0	7	0	4	0	2	4	9	4	0	0	13	24
17:15:00	8	0	0	0	8	1	7	0	7	8	7	10	0	0	17	33
Grand Total	27	3	0	0	30	2	22	0	19	24	31	31	0	0	62	116
Approach%	90%	10%	0%		-	8.3%	91.7%	0%		-	50%	50%	0%		-	-
Totals %	23.3%	2.6%	0%		25.9%	1.7%	19%	0%		20.7%	26.7%	26.7%	0%		53.4%	-
PHF	0.84	0.75	0		0.94	0.5	0.79	0		0.75	0.86	0.78	0		0.82	-
Heavy	0	0	0		0	0	0	0		0	0	1	0		1	
Heavy %	0%	0%	0%		0%	0%	0%	0%		0%	0%	3.2%	0%		1.6%	-
Lights	23	3	0		26	2	22	0		24	31	30	0		61	
Lights %	85.2%	100%	0%		86.7%	100%	100%	0%		100%	100%	96.8%	0%		98.4%	-
Single-Unit Trucks	0	0	0		0	0	0	0		0	0	1	0		1	-
Single-Unit Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	3.2%	0%		1.6%	-
Buses	0	0	0		0	0	0	0		0	0	0	0		0	-
Buses %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Bicycles on Road	4	0	0		4	0	0	0		0	0	0	0		0	-
Bicycles on Road %	14.8%	0%	0%		13.3%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	0	-	-	-	-	19	-	-	-	-	0	-	-
Pedestrians%	-	-	-	0%		-	-	-	100%		-	-	-	0%		-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-



Peak Hour: 08:30 AM - 09:30 AM Weather: Scattered Clouds (1.5 °C) Legend: ### (#.# %) [#.##] TOTAL VEHICLES (HEAVY %) [PHF] Maple Bridge Trait Oaksid 10.801 (16.7%) 12 Oen 1377.7% (0.81) 21 (9.5%) (0.75) Bicycles on Crosswalk Pedestrians N 0 0 s 0 0 Е 0 9 (D) mapbox © Mapbox, © OpenStreetMap





Bicycle %

Turning Movement Count Location Name: REGIONAL RD 8 & CENTRE RD Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					1	Turning M	lovemen	t Count (5 . REGI	ONAL RD 8 & CEN	ITRE RD)						
Start Time				roach RE RD					oroach NAL RD 8					proach NAL RD 8		Int. Total (15 min)	Int. Tot (1 hr)
Start Time	Right N:W	Left N:E	U-Turn N:N	Peds N:	Approach Total	Right E:N	Thru E:W	U-Turn E:E	Peds E:	Approach Total	Thru W:E	Left W:N	U-Turn W:W	Peds W:	Approach Total		
07:00:00	13	4	0	2	17	1	27	0	0	28	7	1	0	0	8	53	
07:15:00	12	3	0	1	15	1	36	0	0	37	12	3	0	0	15	67	
07:30:00	14	3	0	0	17	0	26	0	0	26	21	2	0	0	23	66	
07:45:00	11	7	0	3	18	2	45	0	0	47	24	3	0	0	27	92	278
08:00:00	20	9	0	0	29	1	33	0	0	34	32	3	0	0	35	98	323
08:15:00	13	7	0	1	20	3	34	0	0	37	25	3	0	0	28	85	341
08:30:00	27	5	0	2	32	6	65	0	0	71	35	7	0	0	42	145	420
08:45:00	11	18	0	2	29	5	55	0	0	60	73	8	0	0	81	170	498
09:00:00	9	5	0	1	14	5	31	0	0	36	43	9	0	0	52	102	502
09:15:00	7	6	0	1	13	3	34	0	0	37	41	7	0	0	48	98	515
09:30:00	8	12	0	7	20	5	18	0	0	23	40	4	0	1	44	87	457
09:45:00	11	5	0	9	16	4	24	0	0	28	37	4	0	0	41	85	372
BREAK	(
16:00:00	17	5	0	5	22	9	34	0	0	43	52	17	0	0	69	134	
16:15:00	8	5	0	5	13	8	48	0	0	56	47	15	0	0	62	131	
16:30:00	12	7	0	4	19	10	42	0	0	52	48	24	0	2	72	143	
16:45:00	10	9	0	4	19	11	48	0	0	59	59	15	0	1	74	152	560
17:00:00	12	3	0	1	15	15	48	0	0	63	60	20	0	0	80	158	584
17:15:00	6	8	0	6	14	3	37	0	0	40	56	20	0	3	76	130	583
17:30:00	9	5	0	3	14	9	38	0	0	47	48	13	0	0	61	122	562
17:45:00	7	5	0	1	12	9	30	0	0	39	69	19	0	0	88	139	549
18:00:00	7	1	0	1	8	10	35	0	0	45	42	8	0	2	50	103	494
18:15:00	7	4	0	2	11	5	27	0	0	32	40	7	0	1	47	90	454
18:30:00	4	6	0	7	10	8	26	0	0	34	35	10	0	0	45	89	421
18:45:00	11	5	0	1	16	3	30	0	0	33	42	10	0	0	52	101	383
Grand Total	266	147	0	69	413	136	871	0	0	1007	988	232	0	10	1220	2640	-
Approach%	64.4%	35.6%	0%		-	13.5%	86.5%	0%		-	81%	19%	0%		-	-	-
Totals %	10.1%	5.6%	0%		15.6%	5.2%	33%	0%		38.1%	37.4%	8.8%	0%		46.2%	-	-
Heavy	5	2	0		-	2	29	0		-	47	3	0		-	-	-
Heavy %	1.9%	1.4%	0%		-	1.5%	3.3%	0%		-	4.8%	1.3%	0%		-	-	-
Bicycles	-	-	-		-	-	-	-		-	-	-	-		-	-	-

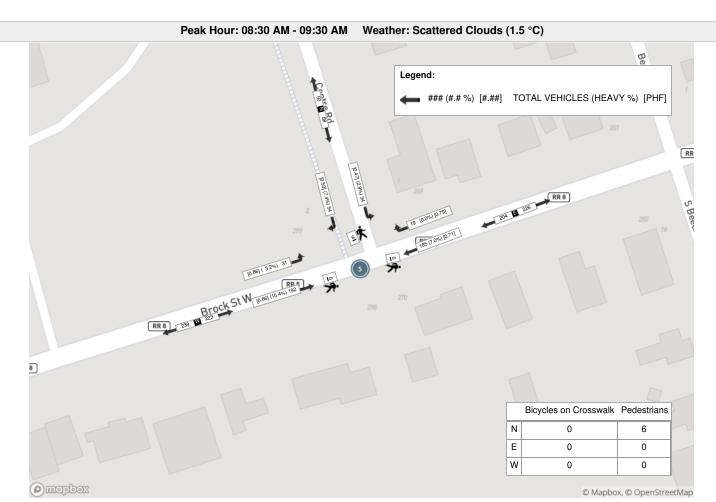
Turning Movement Count Location Name: REGIONAL RD 8 & CENTRE RD Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					Peak Hour: 08:30	AM - 09:	30 AM	Weathe	r: Scatte	ered Clouds (1.5 °	C)					
Start Time				roach RE RD					oroach NAL RD 8				W App REGION	oroach NAL RD 8		Int. Total (15 min)
	Right	Left	U-Turn	Peds	Approach Total	Right	Thru	U-Turn	Peds	Approach Total	Thru	Left	U-Turn	Peds	Approach Total	
08:30:00	27	5	0	2	32	6	65	0	0	71	35	7	0	0	42	145
08:45:00	11	18	0	2	29	5	55	0	0	60	73	8	0	0	81	170
09:00:00	9	5	0	1	14	5	31	0	0	36	43	9	0	0	52	102
09:15:00	7	6	0	1	13	3	34	0	0	37	41	7	0	0	48	98
Grand Total	54	34	0	6	88	19	185	0	0	204	192	31	0	0	223	515
Approach%	61.4%	38.6%	0%		-	9.3%	90.7%	0%		-	86.1%	13.9%	0%		-	-
Totals %	10.5%	6.6%	0%		17.1%	3.7%	35.9%	0%		39.6%	37.3%	6%	0%		43.3%	-
PHF	0.5	0.47	0		0.69	0.79	0.71	0		0.72	0.66	0.86	0		0.69	-
Heavy	4	1	0		5	0	13	0		13	20	1	0		21	
Heavy %	7.4%	2.9%	0%		5.7%	0%	7%	0%		6.4%	10.4%	3.2%	0%		9.4%	-
Lights	50	33	0		83	19	172	0		191	172	30	0		202	
Lights %	92.6%	97.1%	0%		94.3%	100%	93%	0%		93.6%	89.6%	96.8%	0%		90.6%	-
Single-Unit Trucks	1	0	0		1	0	1	0		1	5	1	0		6	-
Single-Unit Trucks %	1.9%	0%	0%		1.1%	0%	0.5%	0%		0.5%	2.6%	3.2%	0%		2.7%	-
Buses	3	1	0		4	0	9	0		9	12	0	0		12	-
Buses %	5.6%	2.9%	0%		4.5%	0%	4.9%	0%		4.4%	6.3%	0%	0%		5.4%	-
Articulated Trucks	0	0	0		0	0	3	0		3	3	0	0		3	-
Articulated Trucks %	0%	0%	0%		0%	0%	1.6%	0%		1.5%	1.6%	0%	0%		1.3%	-
Pedestrians	-	-	-	6	-	-	-	-	0	-	-	-	-	0	-	-
Pedestrians%	-	-	-	100%		-	-	-	0%		-	-	-	0%		-
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Bicycles on Crosswalk%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-

Turning Movement Count Location Name: REGIONAL RD 8 & CENTRE RD Date: Wed, Oct 28, 2020 Deployment Lead: Theo Daglis

					Peak Hour: 04:1	5 PM - 0	5:15 PM	Weath	er: Few	Clouds (10.39 °C)						
Start Time				proach TRE RD				E App REGION	roach IAL RD 8					proach NAL RD 8		Int. Total (15 min)
	Right	Left	U-Turn	Peds	Approach Total	Right	Thru	U-Turn	Peds	Approach Total	Thru	Left	U-Turn	Peds	Approach Total	
16:15:00	8	5	0	5	13	8	48	0	0	56	47	15	0	0	62	131
16:30:00	12	7	0	4	19	10	42	0	0	52	48	24	0	2	72	143
16:45:00	10	9	0	4	19	11	48	0	0	59	59	15	0	1	74	152
17:00:00	12	3	0	1	15	15	48	0	0	63	60	20	0	0	80	158
Grand Total	42	24	0	14	66	44	186	0	0	230	214	74	0	3	288	584
Approach%	63.6%	36.4%	0%		-	19.1%	80.9%	0%		-	74.3%	25.7%	0%		-	-
Totals %	7.2%	4.1%	0%		11.3%	7.5%	31.8%	0%		39.4%	36.6%	12.7%	0%		49.3%	-
PHF	0.88	0.67	0		0.87	0.73	0.97	0		0.91	0.89	0.77	0		0.9	-
Heavy	1	0	0		1	1	3	0		4	4	0	0		4	
Heavy %	2.4%	0%	0%		1.5%	2.3%	1.6%	0%		1.7%	1.9%	0%	0%		1.4%	-
Lights	41	24	0		65	43	183	0		226	210	74	0		284	
Lights %	97.6%	100%	0%		98.5%	97.7%	98.4%	0%		98.3%	98.1%	100%	0%		98.6%	-
Single-Unit Trucks	1	0	0		1	0	2	0		2	3	0	0		3	-
Single-Unit Trucks %	2.4%	0%	0%		1.5%	0%	1.1%	0%		0.9%	1.4%	0%	0%		1%	-
Buses	0	0	0		0	1	1	0		2	1	0	0		1	-
Buses %	0%	0%	0%		0%	2.3%	0.5%	0%		0.9%	0.5%	0%	0%		0.3%	-
Articulated Trucks	0	0	0		0	0	0	0		0	0	0	0		0	-
Articulated Trucks %	0%	0%	0%		0%	0%	0%	0%		0%	0%	0%	0%		0%	-
Pedestrians	-	-	-	11	-	-	-	-	0	-	-	-	-	2	-	-
Pedestrians%	-	-	-	64.7%		-	-	-	0%		-	-	-	11.8%		-
Bicycles on Crosswalk	-	-	-	3	-	-	-	-	0	-	-	-	-	1	-	-
Bicycles on Crosswalk%	-	-	-	17.6%		-	-	-	0%		-	-	-	5.9%		-









Appendix C

Existing Traffic Level of Service Calculations

•				
Intersection				
Intersection Delay, s/veh	6.9			
Intersection LOS	А			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	182	372	156	142
Demand Flow Rate, veh/h	198	377	158	148
Vehicles Circulating, veh/h	302	85	192	376
Vehicles Exiting, veh/h	222	265	308	86
Follow-Up Headway, s	3.186	3.186	3.186	3.186
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	7.3	7.3	5.6	6.9
Approach LOS	А	А	А	А
Lane	Left	Left	Left	Left
Designated Moves	LTR	LTR	LTR	LTR
Assumed Moves	LTR	LTR	LTR	LTR
RT Channelized				
Lane Util	1.000	1.000	1.000	1.000
Critical Headway, s	5.193	5.193	5.193	5.193
Entry Flow, veh/h	198	377	158	148
Cap Entry Lane, veh/h	835	1038	933	776
Entry HV Adj Factor	0.920	0.986	0.985	0.961
Flow Entry, veh/h	182	372	156	142
Cap Entry, veh/h	768	1023	918	746
V/C Ratio	0.237	0.363	0.169	0.191
Control Delay, s/veh	7.3	7.3	5.6	6.9
LOS	А	A	A	А
95th %tile Queue, veh	1	2	1	1

	•	→	•	•	\	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	†	1		¥	
Traffic Volume (veh/h)	26	335	361	29	63	101
Future Volume (Veh/h)	26	335	361	29	63	101
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	31	399	430	35	75	120
Pedestrians					13	
Lane Width (m)					3.5	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	478				922	460
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	478				922	460
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				74	80
cM capacity (veh/h)	1083				291	599
Direction, Lane #	EB 1	EB 2	WB 1	SB 1		
Volume Total	31	399	465	195		
Volume Left	31	0	0	75		
Volume Right	0	0	35	120		
cSH	1083	1700	1700	425		
Volume to Capacity	0.03	0.23	0.27	0.46		
Queue Length 95th (m)	0.7	0.0	0.0	18.8		
Control Delay (s)	8.4	0.0	0.0	20.4		
Lane LOS	А			С		
Approach Delay (s)	0.6		0.0	20.4		
Approach LOS				С		
Intersection Summary						
Average Delay			3.9			
Intersection Capacity Utilization	ation		38.0%	IC	U Level o	f Service
Analysis Period (min)			15	.0		

	•	•	†	~	>	†
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			4
Traffic Volume (veh/h)	32	9	59	16	3	95
Future Volume (Veh/h)	32	9	59	16	3	95
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	36	10	66	18	3	107
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	188	75			84	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	188	75			84	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	95	99			100	
cM capacity (veh/h)	797	992			1526	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total						
	46	84	110			
Volume Left	36	0	3			
Volume Right	10	18	0			
cSH	833	1700	1526			
Volume to Capacity	0.06	0.05	0.00			
Queue Length 95th (m)	1.4	0.0	0.0			
Control Delay (s)	9.6	0.0	0.2			
Lane LOS	A		A			
Approach Delay (s)	9.6	0.0	0.2			
Approach LOS	А					
Intersection Summary						
Average Delay			1.9			
Intersection Capacity Utiliza	ation		17.4%	IC	U Level o	f Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	22	30	56	14	14	4	16	28	6	0	66	24
Future Volume (vph)	22	30	56	14	14	4	16	28	6	0	66	24
Peak Hour Factor	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59
Hourly flow rate (vph)	37	51	95	24	24	7	27	47	10	0	112	41
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	183	55	84	153								
Volume Left (vph)	37	24	27	0								
Volume Right (vph)	95	7	10	41								
Hadj (s)	-0.18	0.11	-0.01	-0.01								
Departure Headway (s)	4.4	4.8	4.6	4.5								
Degree Utilization, x	0.22	0.07	0.11	0.19								
Capacity (veh/h)	776	696	727	743								
Control Delay (s)	8.6	8.2	8.2	8.6								
Approach Delay (s)	8.6	8.2	8.2	8.6								
Approach LOS	Α	Α	Α	Α								
Intersection Summary												
Delay			8.5									
Level of Service			Α									
Intersection Capacity Utilizat	ion		22.9%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

	-	•	•	•	•	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^			4	N/	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	34	24	40	12	44	74
Future Volume (vph)	34	24	40	12	44	74
Peak Hour Factor	0.73	0.73	0.73	0.73	0.73	0.73
Hourly flow rate (vph)	47	33	55	16	60	101
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total (vph)	80	71	161			
Volume Left (vph)	0	55	60			
Volume Right (vph)	33	0	101			
Hadj (s)	-0.07	0.29	-0.22			
Departure Headway (s)	4.3	4.6	4.0			
Degree Utilization, x	0.09	0.09	0.18			
Capacity (veh/h)	810	739	858			
Control Delay (s)	7.7	8.1	7.9			
Approach Delay (s)	7.7	8.1	7.9			
Approach LOS	А	Α	Α			
Intersection Summary						
Delay			7.9			
Level of Service			Α			
Intersection Capacity Utiliz	zation		24.4%	IC	U Level c	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			4
Traffic Volume (veh/h)	52	2	26	42	4	24
Future Volume (Veh/h)	52	2	26	42	4	24
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	68	3	34	55	5	31
Pedestrians	9					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	1					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	112	70			98	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	112	70			98	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	92	100			100	
cM capacity (veh/h)	871	990			1497	
Direction, Lane #	WD 1	NB 1	SB 1			
Volume Total	WB 1 71	89	36			
	68					
Volume Left		0	5			
Volume Right	3	55	1407			
CSH Valume to Conseitu	876	1700	1497			
Volume to Capacity	0.08	0.05	0.00			
Queue Length 95th (m)	2.1	0.0	0.1			
Control Delay (s)	9.5	0.0	1.1			
Lane LOS	A	0.0	A			
Approach Delay (s)	9.5	0.0	1.1			
Approach LOS	А					
Intersection Summary						
Average Delay			3.6			
Intersection Capacity Utili	zation		16.8%	IC	CU Level o	f Service
Analysis Period (min)			15			

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Intersection				
Intersection Delay, s/veh	12.1			
Intersection LOS	В			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	286	224	600	118
Demand Flow Rate, veh/h	292	229	610	120
Vehicles Circulating, veh/h	181	325	273	242
Vehicles Exiting, veh/h	181	558	200	312
Follow-Up Headway, s	3.186	3.186	3.186	3.186
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	7.2	7.7	17.5	5.5
Approach LOS	А	А	С	A
Lane	Left	Left	Left	Left
Designated Moves	LTR	LTR	LTR	LTR
Assumed Moves	LTR	LTR	LTR	LTR
RT Channelized				
Lane Util	1.000	1.000	1.000	1.000
Critical Headway, s	5.193	5.193	5.193	5.193
Entry Flow, veh/h	292	229	610	120
Cap Entry Lane, veh/h	943	816	860	887
Entry HV Adj Factor	0.978	0.976	0.984	0.982
Flow Entry, veh/h	286	224	600	118
Cap Entry, veh/h	922	797	846	871
V/C Ratio	0.310	0.280	0.709	0.135
Control Delay, s/veh	7.2	7.7	17.5	5.5
LOS	Α	A	С	А
95th %tile Queue, veh	1	1	6	0

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	^	1>		W	
Traffic Volume (veh/h)	80	418	348	56	60	25
Future Volume (Veh/h)	80	418	348	56	60	25
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	91	475	395	64	68	28
Pedestrians					4	
Lane Width (m)					3.5	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	463				1088	431
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	463				1088	431
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	92				69	96
cM capacity (veh/h)	1105				220	627
Direction, Lane #	EB 1	EB 2	WB 1	SB 1		
Volume Total	91	475	459	96		
Volume Left	91	0	0	68		
Volume Right	0	0	64	28		
cSH	1105	1700	1700	272		
Volume to Capacity	0.08	0.28	0.27	0.35		
Queue Length 95th (m)	2.1	0.0	0.0	12.3		
Control Delay (s)	8.5	0.0	0.0	25.3		
Lane LOS	А			D		
Approach Delay (s)	1.4		0.0	25.3		
Approach LOS				D		
Intersection Summary						
Average Delay			2.9			
Intersection Capacity Utili	zation		41.1%	IC	U Level c	f Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ»			4
Traffic Volume (veh/h)	32	6	245	47	7	79
Future Volume (Veh/h)	32	6	245	47	7	79
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	34	6	263	51	8	85
Pedestrians			1			
Lane Width (m)			3.5			
Walking Speed (m/s)			1.2			
Percent Blockage			0			
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	390	288			314	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	390	288			314	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	94	99			99	
cM capacity (veh/h)	607	755			1258	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	40	314	93			
Volume Left	34	0	8			
Volume Right	6	51	0			
cSH	626	1700	1258			
Volume to Capacity	0.06	0.18	0.01			
Queue Length 95th (m)	1.6	0.0	0.2			
Control Delay (s)	11.1	0.0	0.7			
Lane LOS	В	0.0	Α			
Approach Delay (s)	11.1	0.0	0.7			
Approach LOS	В	0.0	0.1			
	U					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utiliza	ation		25.7%	IC	U Level c	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	22	20	34	18	18	10	44	92	22	8	66	26
Future Volume (vph)	22	20	34	18	18	10	44	92	22	8	66	26
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	24	22	38	20	20	11	49	102	24	9	73	29
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	84	51	175	111								
Volume Left (vph)	24	20	49	9								
Volume Right (vph)	38	11	24	29								
Hadj (s)	-0.21	-0.05	-0.01	-0.14								
Departure Headway (s)	4.4	4.6	4.3	4.3								
Degree Utilization, x	0.10	0.07	0.21	0.13								
Capacity (veh/h)	758	723	799	800								
Control Delay (s)	7.9	7.9	8.5	7.9								
Approach Delay (s)	7.9	7.9	8.5	7.9								
Approach LOS	Α	Α	Α	Α								
Intersection Summary												
Delay			8.1									
Level of Service			Α									
Intersection Capacity Utilizati	on		29.9%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			4	W	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	46	16	38	48	28	40
Future Volume (vph)	46	16	38	48	28	40
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86
Hourly flow rate (vph)	53	19	44	56	33	47
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total (vph)	72	100	80			
Volume Left (vph)	0	44	33			
Volume Right (vph)	19	0	47			
Hadj (s)	-0.16	0.09	-0.27			
Departure Headway (s)	4.0	4.2	4.0			
Degree Utilization, x	0.08	0.12	0.09			
Capacity (veh/h)	871	830	856			
Control Delay (s)	7.4	7.8	7.4			
Approach Delay (s)	7.4	7.8	7.4			
Approach LOS	Α	Α	Α			
Intersection Summary						
Delay			7.5			
Level of Service			Α			
Intersection Capacity Utiliz	zation		23.4%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	¥		₽			4	Ī
Traffic Volume (veh/h)	44	4	62	62	6	54	
Future Volume (Veh/h)	44	4	62	62	6	54	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	
Hourly flow rate (vph)	47	4	66	66	6	57	
Pedestrians	19						
Lane Width (m)	3.5						
Walking Speed (m/s)	1.2						
Percent Blockage	2						
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	187	118			151		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	187	118			151		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	94	100			100		
cM capacity (veh/h)	791	925			1420		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	51	132	63				
Volume Left	47	0	6				
Volume Right	4	66	0				
cSH	800	1700	1420				
Volume to Capacity	0.06	0.08	0.00				
Queue Length 95th (m)	1.6	0.0	0.1				
Control Delay (s)	9.8	0.0	0.7				
Lane LOS	A		Α				
Approach Delay (s)	9.8	0.0	0.7				
Approach LOS	А						
Intersection Summary							
Average Delay			2.2				
Intersection Capacity Utiliz	zation		20.5%	IC	U Level o	of Service	
Analysis Period (min)			15				

Appendix D

Future Background Level of Service Calculations

3: Concession 6 & Regional Road 8/Brock Street W

Intersection				
Intersection Delay, s/veh	7.7			
Intersection LOS	А			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	210	417	163	172
Demand Flow Rate, veh/h	229	424	166	179
Vehicles Circulating, veh/h	333	93	223	423
Vehicles Exiting, veh/h	269	296	339	94
Follow-Up Headway, s	3.186	3.186	3.186	3.186
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	8.2	8.1	5.9	7.9
Approach LOS	A	А	А	A
Lane	Left	Left	Left	Left
Designated Moves	LTR	LTR	LTR	LTR
Assumed Moves	LTR	LTR	LTR	LTR
RT Channelized				
Lane Util	1.000	1.000	1.000	1.000
Critical Headway, s	5.193	5.193	5.193	5.193
Entry Flow, veh/h	229	424	166	179
Cap Entry Lane, veh/h	810	1030	904	740
Entry HV Adj Factor	0.918	0.984	0.984	0.961
Flow Entry, veh/h	210	417	163	172
Cap Entry, veh/h	744	1013	890	711
V/C Ratio	0.283	0.412	0.184	0.242
Control Delay, s/veh	8.2	8.1	5.9	7.9
LOS	Α	А	А	А
95th %tile Queue, veh	1	2	1	1

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	†	1		¥	
Traffic Volume (veh/h)	31	393	423	29	74	131
Future Volume (Veh/h)	31	393	423	29	74	131
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	37	468	504	35	88	156
Pedestrians					13	
Lane Width (m)					3.5	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	552				1076	534
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	552				1076	534
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	96				62	71
cM capacity (veh/h)	1017				233	544
Direction, Lane #	EB 1	EB 2	WB 1	SB 1		
Volume Total	37	468	539	244		
Volume Left	37	0	0	88		
Volume Right	0	0	35	156		
cSH	1017	1700	1700	367		
Volume to Capacity	0.04	0.28	0.32	0.66		
Queue Length 95th (m)	0.9	0.0	0.0	36.6		
Control Delay (s)	8.7	0.0	0.0	32.2		
Lane LOS	А			D		
Approach Delay (s)	0.6		0.0	32.2		
Approach LOS				D		
Intersection Summary						
Average Delay			6.3			
Intersection Capacity Utiliz	ation		44.6%	IC	U Level o	f Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	25	35	56	14	16	4	16	38	6	0	90	36
Future Volume (vph)	25	35	56	14	16	4	16	38	6	0	90	36
Peak Hour Factor	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59
Hourly flow rate (vph)	42	59	95	24	27	7	27	64	10	0	153	61
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	196	58	101	214								
Volume Left (vph)	42	24	27	0								
Volume Right (vph)	95	7	10	61								
Hadj (s)	-0.15	0.12	-0.01	-0.02								
Departure Headway (s)	4.6	5.0	4.8	4.6								
Degree Utilization, x	0.25	0.08	0.13	0.28								
Capacity (veh/h)	730	651	701	732								
Control Delay (s)	9.1	8.5	8.5	9.4								
Approach Delay (s)	9.1	8.5	8.5	9.4								
Approach LOS	Α	А	Α	Α								
Intersection Summary												
Delay			9.1									
Level of Service			Α									
Intersection Capacity Utilization 29.8%		IC	U Level o	of Service			Α					
Analysis Period (min)			15									

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)			ર્ન	W		
Sign Control	Stop			Stop	Stop		
Traffic Volume (vph)	43	24	40	26	52	87	
Future Volume (vph)	43	24	40	26	52	87	
Peak Hour Factor	0.73	0.73	0.73	0.73	0.73	0.73	
Hourly flow rate (vph)	59	33	55	36	71	119	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total (vph)	92	91	190				
Volume Left (vph)	0	55	71				
Volume Right (vph)	33	0	119				
Hadj (s)	-0.04	0.22	-0.22				
Departure Headway (s)	4.4	4.7	4.1				
Degree Utilization, x	0.11	0.12	0.22				
Capacity (veh/h)	780	731	838				
Control Delay (s)	8.0	8.3	8.3				
Approach Delay (s)	8.0	8.3	8.3				
Approach LOS	Α	Α	А				
Intersection Summary							
Delay			8.2				
Level of Service			Α				
Intersection Capacity Utilization	ation		26.2%	IC	U Level o	f Service	
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		î,			4
Traffic Volume (veh/h)	61	2	38	42	4	53
Future Volume (Veh/h)	61	2	38	42	4	53
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	79	3	49	55	5	69
Pedestrians	9					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	1					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	164	86			113	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	164	86			113	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	90	100			100	
cM capacity (veh/h)	813	972			1478	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	82	104	74			
Volume Left	79	0	5			
Volume Right	3	55	0			
cSH	818	1700	1478			
Volume to Capacity	0.10	0.06	0.00			
Queue Length 95th (m)	2.7	0.00	0.00			
• • • • • • • • • • • • • • • • • • • •	9.9					
Control Delay (s)		0.0	0.5			
Lane LOS	A	0.0	A			
Approach LOS	9.9	0.0	0.5			
Approach LOS	А					
Intersection Summary						
Average Delay			3.3			
Intersection Capacity Utiliz	zation		17.4%	IC	U Level c	f Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			4
Traffic Volume (veh/h)	25	8	30	8	3	28
Future Volume (Veh/h)	25	8	30	8	3	28
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	32	10	39	10	4	36
Pedestrians	9					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	1					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	97	53			58	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	97	53			58	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	96	99			100	
cM capacity (veh/h)	888	1013			1548	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	42	49	40			
Volume Left	32	0	40			
Volume Right	10	1700	1540			
cSH Valume to Canadity	915	1700	1548			
Volume to Capacity	0.05	0.03	0.00			
Queue Length 95th (m)	1.2	0.0	0.1			
Control Delay (s)	9.1	0.0	0.8			
Lane LOS	A	0.0	A			
Approach Delay (s)	9.1	0.0	0.8			
Approach LOS	А					
Intersection Summary						
Average Delay			3.2			
Intersection Capacity Utiliz	zation		15.9%	IC	:U Level o	f Service
Analysis Period (min)			15			

3: Concession 6 & Regional Road 8/Brock Street W

Intersection				
Intersection Delay, s/veh	16.3			
Intersection LOS	С			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	343	253	654	139
Demand Flow Rate, veh/h	351	260	665	142
Vehicles Circulating, veh/h	203	380	332	273
Vehicles Exiting, veh/h	212	617	222	367
Follow-Up Headway, s	3.186	3.186	3.186	3.186
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	8.3	8.9	25.6	5.9
Approach LOS	А	А	D	A
Lane	Left	Left	Left	Left
Designated Moves	LTR	LTR	LTR	LTR
Assumed Moves	LTR	LTR	LTR	LTR
RT Channelized				
Lane Util	1.000	1.000	1.000	1.000
Critical Headway, s	5.193	5.193	5.193	5.193
Entry Flow, veh/h	351	260	665	142
Cap Entry Lane, veh/h	922	773	811	860
Entry HV Adj Factor	0.977	0.975	0.983	0.980
Flow Entry, veh/h	343	253	654	139
Cap Entry, veh/h	901	753	797	843
V/C Ratio	0.381	0.336	0.820	0.165
Control Delay, s/veh	8.3	8.9	25.6	5.9
LOS	Α	А	D	А
95th %tile Queue, veh	2	1	9	1

BBL BBT WBT WBR SBL SBR		•	→	•	•	\	1
Lane Configurations	Movement	EBL	EBT	WBT	WBR	SBL	SBR
Traffic Volume (veh/h) 89 490 408 56 70 39 Future Volume (Veh/h) 89 490 408 56 70 39 Sign Control Free Free Stop Corade 0% 0% 0% Peak Hour Factor 0.93 1.93 9.93 9.93 9.93 9.93 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>							
Future Volume (Veh/h) 89 490 408 56 70 39 Sign Control Free Free Stop Grade 0% 0% 0% 0% Peak Hour Factor 0.93 0.93 0.93 0.93 0.93 Hourly flow rate (vph) 96 527 439 60 75 42 Pedestrians 4 Lane Width (m) 3.5 Walking Speed (m/s) 1.2 Percent Blockage 0 Median type None None Median type None None Median type S03 1192 473 VC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage (s) If (s) 2.2 3.5 3.3 p0 queue free % 91 60 93 CM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume Right 0 0 0 60 42 cSH 1068 1700 1700 251 Volume Right 0 0 0 60 42 cSH 108 133 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6					56		39
Sign Control Free Grade Free Owner Free Owner Stop Owner Grade 0% 0% 0% Peak Hour Factor 0.93							
Grade 0% 0% 0% Peak Hour Factor 0.93							
Peak Hour Factor							
Hourly flow rate (vph) 96 527 439 60 75 42 Pedestrians 4 4 Lane Width (m) 3.5 Walking Speed (m/s) 1.2 Percent Blockage 0 Right turn flare (veh) Median type None None Median storage veh Upstream signal (m) pX, platoon unblocked vC, conflicting volume 503 1192 473 vC1, stage 1 conf vol vC2, stage 2 conf vol vC4, unblocked vol 503 1192 473 tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Cueue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6		0.93			0.93		0.93
Pedestrians							
Lane Width (m) 3.5 Walking Speed (m/s) 1.2 Percent Blockage 0 Right turn flare (veh) None Median storage veh) Upstream signal (m) Upx, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) 2.2 3.5 3.3 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0							
Walking Speed (m/s) 1.2 Percent Blockage 0 Right turn flare (veh) None Median type None Median storage veh) Upstream signal (m) Dystream signal (m) 503 VC1, stage 1 conf vol vC2, stage 2 conf vol VC2, stage 2 conf vol vCu, unblocked vol VC3, stage (s) 1192 473 tF (s) 2.2 3.5 3.3 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3							
Percent Blockage 0 Right turn flare (veh) None None Median type None None Median storage veh) Upstream signal (m) None My px, platoon unblocked vC, conflicting volume 503 1192 473 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, stage (s) 4.1 6.4 6.2 6.2 6.4 6.2 6.2 6.4 6.2 6.2 6.4 6.2 6.2 6.2 3.5 3.3 9.3							
Right turn flare (veh) Median type Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume vC2, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 Big 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 1.3 0.0 31.3 Approach Delay (s) 1.13 0.0 31.3 Approach LOS Intersection Summary Average Delay None None None None None None None Non							
Median type None None Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume 503 1192 473 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 503 1192 473 tC, single (s) 4.1 6.4 6.2 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume 503 1192 473 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 503 1192 473 tC, single (s) 4.1 6.4 6.2 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach LOS D <td></td> <td></td> <td>None</td> <td>None</td> <td></td> <td></td> <td></td>			None	None			
Upstream signal (m) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol vCu, unblocked vol single (s) tF (s) 2.2 3.5 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) Approach Delay (s) Approach LOS Intersection Summary Average Delay 30 1192 473 473 1192							
pX, platoon unblocked vC, conflicting volume 503 1192 473 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 503 1192 473 tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 pO queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
VC, conflicting volume 503 1192 473 vC1, stage 1 conf vol VC2, stage 2 conf vol VCU, unblocked vol 503 1192 473 tC, single (s) 4.1 6.4 6.2 8.3 3.3 9.3 9.3 9.3 8.8 9.3 8.8 9.3 8.8 9.3 9.3 9.2 9.4 7.2 9.2 1.3 9.2 9.4 7.2 9.2 1.2 9.2							
vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 503 1192 473 tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) 50 3.5 3.3 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary		503				1192	473
vC2, stage 2 conf vol vCu, unblocked vol 503 1192 473 tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) 50 3.5 3.3 pO queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach LOS D Intersection Summary Average Delay 3.6							
vCu, unblocked vol 503 1192 473 tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) 503 3.5 3.5 3.3 pC (s) 2.2 3.5 3.3 <							
tC, single (s) tC, 2 stage (s) tF (s)		503				1192	473
tC, 2 stage (s) tF (s)							
tF (s) 2.2 3.5 3.3 p0 queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
p0 queue free % 91 60 93 cM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6		2.2				3.5	3.3
CM capacity (veh/h) 1068 189 593 Direction, Lane # EB 1 EB 2 WB 1 SB 1 Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
Volume Total 96 527 499 117 Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6	Direction, Lane #	EB 1	EB 2	WB 1	SB 1		
Volume Left 96 0 0 75 Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
Volume Right 0 0 60 42 cSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
CSH 1068 1700 1700 251 Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
Volume to Capacity 0.09 0.31 0.29 0.47 Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
Queue Length 95th (m) 2.4 0.0 0.0 18.5 Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
Control Delay (s) 8.7 0.0 0.0 31.3 Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
Lane LOS A D Approach Delay (s) 1.3 0.0 31.3 Approach LOS D Intersection Summary Average Delay 3.6							
Approach Delay (s) Approach LOS Intersection Summary Average Delay 3.6			5.0	0.0			
Approach LOS D Intersection Summary Average Delay 3.6				0.0			
Average Delay 3.6							
Average Delay 3.6	Intersection Summary						
				3.6			
Intersection Capacity Utilization 46.1% ICU Level of Service	Intersection Capacity Utiliz	zation		46.1%	IC	III evel d	f Service
Analysis Period (min) 15		Lation			10	LOVOIC	, Joi vice

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	¥		f)			4	
Traffic Volume (veh/h)	45	7	287	57	7	93	
Future Volume (Veh/h)	45	7	287	57	7	93	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Hourly flow rate (vph)	48	8	309	61	8	100	
Pedestrians			1				
Lane Width (m)			3.5				
Walking Speed (m/s)			1.2				
Percent Blockage			0				
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	456	340			370		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	456	340			370		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	91	99			99		
cM capacity (veh/h)	556	707			1200		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	56	370	108				
Volume Left	48	0	8				
Volume Right	8	61	0				
cSH	573	1700	1200				
Volume to Capacity	0.10	0.22	0.01				
Queue Length 95th (m)	2.6	0.0	0.2				
Control Delay (s)	12.0	0.0	0.6				
Lane LOS	В	3.0	A				
Approach Delay (s)	12.0	0.0	0.6				
Approach LOS	В	0.0	0.0				
Intersection Summary							
Average Delay			1.4				
Intersection Capacity Utiliz	zation		28.6%	IC	:U Level c	f Service	
Analysis Period (min)	Lation		15	10	O LOVEI C	JUI VICE	
Analysis Fenou (IIIII)			13				

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	32	23	34	18	21	10	44	127	22	8	87	34
Future Volume (vph)	32	23	34	18	21	10	44	127	22	8	87	34
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	36	26	38	20	23	11	49	141	24	9	97	38
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	100	54	214	144								
Volume Left (vph)	36	20	49	9								
Volume Right (vph)	38	11	24	38								
Hadj (s)	-0.16	-0.05	0.00	-0.15								
Departure Headway (s)	4.6	4.8	4.4	4.4								
Degree Utilization, x	0.13	0.07	0.26	0.18								
Capacity (veh/h)	713	683	778	778								
Control Delay (s)	8.3	8.2	9.0	8.3								
Approach Delay (s)	8.3	8.2	9.0	8.3								
Approach LOS	Α	Α	Α	Α								
Intersection Summary												
Delay			8.6									
Level of Service			Α									
Intersection Capacity Utilizat	tion		37.5%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1→			ર્ન	W	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	64	16	38	64	33	47
Future Volume (vph)	64	16	38	64	33	47
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86
Hourly flow rate (vph)	74	19	44	74	38	55
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total (vph)	93	118	93			
Volume Left (vph)	0	44	38			
Volume Right (vph)	19	0	55			
Hadj (s)	-0.12	0.07	-0.27			
Departure Headway (s)	4.1	4.3	4.1			
Degree Utilization, x	0.11	0.14	0.11			
Capacity (veh/h)	850	820	833			
Control Delay (s)	7.6	8.0	7.6			
Approach Delay (s)	7.6	8.0	7.6			
Approach LOS	Α	Α	Α			
Intersection Summary						
Delay			7.7			
Level of Service			А			
Intersection Capacity Utilization	ation		24.8%	IC	U Level c	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			4
Traffic Volume (veh/h)	52	5	102	62	6	81
Future Volume (Veh/h)	52	5	102	62	6	81
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	55	5	109	66	6	86
Pedestrians	19					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	2					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	259	161			194	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	259	161			194	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	92	99			100	
cM capacity (veh/h)	720	876			1370	
	WB 1	NB 1	SB 1			
Direction, Lane # Volume Total	60	175	92			
Volume Left	55	0	6			
	5	66	0			
Volume Right cSH	731	1700	1370			
	0.08	0.10	0.00			
Volume to Capacity	2.1					
Queue Length 95th (m)		0.0	0.1			
Control Delay (s)	10.4	0.0	0.5			
Lane LOS	B	0.0	A			
Approach Delay (s)	10.4	0.0	0.5			
Approach LOS	В					
Intersection Summary						
Average Delay			2.1			
Intersection Capacity Utiliz	ation		21.5%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		f)			4
Traffic Volume (veh/h)	18	5	73	29	9	63
Future Volume (Veh/h)	18	5	73	29	9	63
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	19	5	78	31	10	67
Pedestrians	19					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	2					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	200	112			128	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	200	112			128	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	99			99	
cM capacity (veh/h)	776	931			1448	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	24	109	77			
Volume Left	19	0	10			
Volume Right	5	31	0			
cSH	804	1700	1448			
Volume to Capacity	0.03	0.06	0.01			
Queue Length 95th (m)	0.7	0.0	0.2			
Control Delay (s)	9.6	0.0	1.0			
Lane LOS	А		Α			
Approach Delay (s)	9.6	0.0	1.0			
Approach LOS	А					
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utiliz	zation		20.5%	IC	:U Level o	of Service
Analysis Period (min)	-41011		15	10	O LOVEI C	n Jeivie
Analysis Penou (mill)			13			

Appendix E2016 Transportation Tomorrow Survey (TTS) Data Analysis

Mode of Transportation - AM Peak Period

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: Primary travel mode of trip - mode_prime Column: 2006 GTA zone of household - gta06_hhld

Filters:

Primary travel mode of trip - mode_prime In B C D G J M P T U W

and

2006 GTA zone of household - gta06_hhld In 1313 1316 1317 1318

and

Start time of trip - start_time In 600-900

Trip 2016 Table:

Mode of Transportation/Traffic Zones	1313	1316	1317	1318	Total	Percentage
Transit excluding GO rail	10	0	26	21	57	1%
Cycle	0	0	0	23	23	0%
Auto driver	593	459	2160	1325	4537	68%
GO rail only	0	0	162	56	218	3%
Joint GO rail and local transit	0	0	13	0	13	0%
Auto passenger	88	0	335	318	741	11%
Walk	0	27	612	402	1041	16%
Total	691	486	3308	2145	6630	100%

Mode of Transportation - PM Peak Period

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: Primary travel mode of trip - mode_prime Column: 2006 GTA zone of household - gta06_hhld

Filters:

Primary travel mode of trip - mode_prime ln B $\,$ C $\,$ D $\,$ G $\,$ J $\,$ M $\,$ P $\,$ T $\,$ U $\,$ W

and

2006 GTA zone of household - gta06_hhld In 1313 1316 1317 1318

and

Start time of trip - start_time In 1600-1900

Trip 2016 Table:

Mode of Transportation/Traffic Zones	1313	1316	1317	1318	Total	Percentage
Transit excluding GO rail	0	0	0	57	57	1%
Cycle	0	0	48	23	71	1%
Auto driver	852	652	1815	2063	5382	74%
GO rail only	0	0	96	56	152	2%
Joint GO rail and local transit	10	0	13	0	23	0%
Motorcycle	0	0	0	146	146	2%
Auto passenger	226	11	337	542	1116	15%
Taxi passenger	0	0	0	17	17	0%
Walk	0	0	186	94	280	4%
Total	1088	663	2495	2998	7244	100%

Auto Distribution

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: 2006 GTA zone of origin - gta06_orig Column: Planning district of destination - pd_dest

Primary travel mode of trip - mode_prime In D $\,$ M $\,$ P $\,$ T $\,$ U and 2006 GTA zone of origin - gta06_orig In 1313 1316 1317 1318

Start time of trip - start_time In 600-900

Trip 2016

PD 1 of Tor PD 5 of Tor PD 6 of Tor PD 6 of Tor PD 9 of Tor PD 12 of Tor PD 13 of Tor PD 14 of Tor PD 15 of Tor PD 16 of Tor PD 16 of Tor BD 6 of Tor PD 16 of Tor BD 6 of Tor PD 16 of Tor 1317 75 9 95 2% 43 156 12 98 2% 78 1% 20 0% 13 0% 14 0% 18 0% 26 1% 26 1% 119 59 744 204 59 2104 4% 1% 38% 0 44 107 44 2% 1% 140 3% 21 0% 9 0% 250 5% 437 8% 564 10% 10 0% 38 1% 14 0% 7% 2% 2% 100% 3% 3% 1% 1% 1%

Uxbridge 38%
Durham 17%
York 31%
Peel 1%
Toronto 10%
Others 3%
100%

Transit Distribution

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: 2006 GTA zone of origin - gta06_orig Column: Planning district of destination - pd_dest

Filters:

Primary travel mode of trip - mode_prime In G	С	G	J	W
and				
2006 GTA zone of origin - gta06_orig In 1313		1316	1317	1318
and				

Start time of trip - start_time In 600-900

Trip 2016 Table:

	PD 1 of Toronto	Uxbridge	
1316	0	14	
1318	56	443	
	56	457	513
	11%	89%	

Appendix F

Future Total Level of Service Calculations

Intersection				
Intersection Delay, s/veh	9.1			
Intersection LOS	А			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	240	417	179	311
Demand Flow Rate, veh/h	263	424	182	320
Vehicles Circulating, veh/h	381	143	257	423
Vehicles Exiting, veh/h	362	296	387	144
Follow-Up Headway, s	3.186	3.186	3.186	3.186
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	9.4	8.7	6.3	10.9
Approach LOS	А	A	A	В
Lane	Left	Left	Left	Left
Designated Moves	LTR	LTR	LTR	LTR
Assumed Moves	LTR	LTR	LTR	LTR
RT Channelized				
Lane Util	1.000	1.000	1.000	1.000
Critical Headway, s	5.193	5.193	5.193	5.193
Entry Flow, veh/h	263	424	182	320
Cap Entry Lane, veh/h	772	979	874	740
Entry HV Adj Factor	0.914	0.984	0.983	0.972
Flow Entry, veh/h	240	417	179	311
Cap Entry, veh/h	705	964	859	720
V/C Ratio	0.341	0.433	0.208	0.432
Control Delay, s/veh	9.4	8.7	6.3	10.9
LOS	Α	А	А	В
95th %tile Queue, veh	2	2	1	2

	•	→	←	4	\	1
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*		1>		W	
Traffic Volume (veh/h)	31	393	423	34	84	131
Future Volume (Veh/h)	31	393	423	34	84	131
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	33	423	455	37	90	141
Pedestrians					13	
Lane Width (m)					3.5	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	505				976	486
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	505				976	486
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				67	76
cM capacity (veh/h)	1059				269	579
Direction, Lane #	EB 1	EB 2	WB 1	SB 1		
Volume Total	33	423	492	231		
Volume Left	33	0	0	90		
Volume Right	0	0	37	141		
cSH	1059	1700	1700	400		
Volume to Capacity	0.03	0.25	0.29	0.58		
Queue Length 95th (m)	0.8	0.0	0.0	28.1		
Control Delay (s)	8.5	0.0	0.0	25.6		
Lane LOS	А			D		
Approach Delay (s)	0.6		0.0	25.6		
Approach LOS				D		
Intersection Summary						
Average Delay			5.3			
Intersection Capacity Utiliz	zation		45.1%	IC	U Level c	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			ર્ન
Traffic Volume (veh/h)	49	16	110	19	17	235
Future Volume (Veh/h)	49	16	110	19	17	235
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	55	18	124	21	19	264
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	436	134			145	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	436	134			145	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	90	98			99	
cM capacity (veh/h)	568	920			1450	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	73	145	283			
Volume Left	55	0	19			
Volume Right	18	21	0			
cSH	627	1700	1450			
Volume to Capacity	0.12	0.09	0.01			
Queue Length 95th (m)	3.1	0.0	0.3			
Control Delay (s)	11.5	0.0	0.6			
Lane LOS	В		A			
Approach Delay (s)	11.5	0.0	0.6			
Approach LOS	В					
Intersection Summary						
Average Delay			2.0			
Intersection Capacity Utilization	ation		33.9%	IC	:U Level d	f Service
Analysis Period (min)			15	.0	2 20101	
raidiysis i criod (iliil)			13			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	30	35	56	14	16	4	16	43	6	6	104	50
Future Volume (vph)	30	35	56	14	16	4	16	43	6	6	104	50
Peak Hour Factor	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59	0.59
Hourly flow rate (vph)	51	59	95	24	27	7	27	73	10	10	176	85
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	205	58	110	271								
Volume Left (vph)	51	24	27	10								
Volume Right (vph)	95	7	10	85								
Hadj (s)	-0.12	0.12	-0.01	-0.04								
Departure Headway (s)	4.8	5.2	4.9	4.7								
Degree Utilization, x	0.27	0.08	0.15	0.35								
Capacity (veh/h)	696	618	679	727								
Control Delay (s)	9.6	8.7	8.8	10.2								
Approach Delay (s)	9.6	8.7	8.8	10.2								
Approach LOS	Α	Α	Α	В								
Intersection Summary												
Delay			9.6									
Level of Service			Α									
Intersection Capacity Utilizat	tion		24.9%	IC	U Level o	of Service	!		Α			
Analysis Period (min)			15									

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Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations	ĵ.			4	¥			
Sign Control	Stop			Stop	Stop			
Traffic Volume (vph)	43	38	54	26	57	92		
Future Volume (vph)	43	38	54	26	57	92		
Peak Hour Factor	0.73	0.73	0.73	0.73	0.73	0.73		
Hourly flow rate (vph)	59	52	74	36	78	126		
Direction, Lane #	EB 1	WB 1	NB 1					
Volume Total (vph)	111	110	204				Ī	
Volume Left (vph)	0	74	78					
Volume Right (vph)	52	0	126					
Hadj (s)	-0.11	0.25	-0.21					
Departure Headway (s)	4.4	4.7	4.2					
Degree Utilization, x	0.14	0.14	0.24					
Capacity (veh/h)	779	716	814					
Control Delay (s)	8.1	8.5	8.5					
Approach Delay (s)	8.1	8.5	8.5					
Approach LOS	Α	Α	Α					
Intersection Summary								
Delay			8.4					
Level of Service			Α					
Intersection Capacity Utiliz	ation		27.5%	IC	U Level c	of Service		
Analysis Period (min)			15					

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		î,			4
Traffic Volume (veh/h)	61	2	50	42	4	87
Future Volume (Veh/h)	61	2	50	42	4	87
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	79	3	65	55	5	113
Pedestrians	9					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	1					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	224	102			129	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	224	102			129	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	89	100			100	
cM capacity (veh/h)	751	952			1458	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	82	120	118			
Volume Left	79	0	5			
Volume Right	3	55	0			
cSH	757	1700	1458			
Volume to Capacity	0.11	0.07	0.00			
Queue Length 95th (m)	2.9	0.0	0.1			
Control Delay (s)	10.3	0.0	0.3			
Lane LOS	В		А			
Approach Delay (s)	10.3	0.0	0.3			
Approach LOS	В					
Intersection Summary						
Average Delay			2.8			
Intersection Capacity Utiliz	ration		18.0%	IC	U Level o	f Service
Analysis Period (min)	-4.1011		15.076	10	LOVOIO	. 30, 1100
Analysis i Gilou (IIIIII)			13			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	22	37	34	25	11	8	12	30	8	3	28	7
Future Volume (Veh/h)	22	37	34	25	11	8	12	30	8	3	28	7
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	29	48	44	32	14	10	16	39	10	4	36	9
Pedestrians		10			9							
Lane Width (m)		3.5			3.5							
Walking Speed (m/s)		1.2			1.2							
Percent Blockage		1			1							
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	152	148	50	202	148	53	55			58		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	152	148	50	202	148	53	55			58		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	96	93	96	95	98	99	99			100		
cM capacity (veh/h)	777	726	1015	664	726	1013	1550			1548		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	121	56	65	49								
Volume Left	29	32	16	4								
Volume Right	44	10	10	9								
cSH	824	724	1550	1548								
Volume to Capacity	0.15	0.08	0.01	0.00								
Queue Length 95th (m)	4.1	2.0	0.3	0.1								
Control Delay (s)	10.1	10.4	1.9	0.6								
Lane LOS	В	В	Α	А								
Approach Delay (s)	10.1	10.4	1.9	0.6								
Approach LOS	В	В										
Intersection Summary												
Average Delay			6.7									
Intersection Capacity Utilizat	ion		19.9%	IC	CU Level of	of Service			Α			
Analysis Period (min)			15									

Movement		•	4	†	~	/	ţ	_	
Traffic Volume (veh/h) 82 52 98 28 8 170 Future Volume (Veh/h) 82 52 98 28 8 170 Sign Control Stop Free Free Free Grade 0% 0% 0% 0% Peak Hour Factor 0.89 0.89 0.89 0.89 0.89 0.89 Hourly flow rate (vph) 92 58 110 31 9 191 Pedestrians 10 1.2 <th>Movement</th> <th>WBL</th> <th>WBR</th> <th>NBT</th> <th>NBR</th> <th>SBL</th> <th>SBT</th> <th></th> <th></th>	Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Traffic Volume (veh/h) 82 52 98 28 8 170 Future Volume (Veh/h) 82 52 98 28 8 170 Sign Control Stop Free Free Free Grade 0% 0% 0% 0% Peak Hour Factor 0.89 0.89 0.89 0.89 0.89 0.89 Hourly flow rate (vph) 92 58 110 31 9 191 Pedestrians 10 1.2 1 10 1.2 1 10 1.2 1 10 1.2 1.2 1	Lane Configurations	W		f)			4		
Future Volume (Veh/h) Sign Control Stop Free Grade O% O% O% O% O% O% O% Peak Hour Factor None Pedestrians 10 Lane Width (m) 3.5 Walking Speed (m/s) Percent Blockage Right turn flare (veh) Median type None Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC1, stage (s) If (s) Sign Control Stop Free Free Free Free O% O% O% O% O% O% O% O% O%			52		28	8			
Grade 0% 0% 0% 0% 0% Peak Hour Factor 0.89 0.89 0.89 0.89 0.89 0.89 0.89 Hourly flow rate (vph) 92 58 110 31 9 191 Pedestrians 10 Lane Width (m) 3.5 Walking Speed (m/s) 1.2 Percent Blockage 1 Right turn flare (veh) Median type None None Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Right 58 31 0 cSH 724 1700 1431 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Approach LOS B		82	52	98		8	170		
Grade 0% 0% 0% Peak Hour Factor 0.89	Sign Control	Stop		Free			Free		
Hourly flow rate (vph) 92 58 110 31 9 191 Pedestrians 10 Lane Width (m) 3.5 Walking Speed (m/s) 1.2 Percent Blockage 1 Right turn flare (veh) Median type None None Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC4, unblocked vol tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Approach LOS B				0%			0%		
Pedestrians	Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89		
Pedestrians	Hourly flow rate (vph)	92	58	110	31	9	191		
Walking Speed (m/s) 1.2 Percent Blockage 1 Right turn flare (veh) None Median storage veh) None Upstream signal (m) None pX, platoon unblocked VC, conflicting volume vC1, stage 1 conf vol VC2, stage 2 conf vol vC2, stage 2 conf vol VC4, unblocked vol tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) 151 151 151 tC, 2 stage (s) 15 3.5 3.3 2.2 2 p0 queue free % 86 94 99 99 99 99 99 90		10							
Percent Blockage 1 Right turn flare (veh) Median type None None Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume 344 136 151 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 344 136 151 tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Approach LOS B A Approach LOS B	Lane Width (m)	3.5							
Right turn flare (veh) Median type Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) tC, single (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B	Walking Speed (m/s)	1.2							
Right turn flare (veh) Median type Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) tC, single (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B		1							
Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume 344 136 151 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 344 136 151 tC, stage (s) 6.4 6.2 4.1 4.1 tC, 2 stage (s) 5 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B 0.4	Right turn flare (veh)								
Upstream signal (m) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) tF, single (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Approach LOS B	Median type			None			None		
pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 344 136 151 tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B									
vC, conflicting volume 344 136 151 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 344 136 151 tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B 0.0 0.4	Upstream signal (m)								
vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 344 136 151 tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) 5 3.5 3.3 2.2 p0 queue free % 86 94 99 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B A	pX, platoon unblocked								
vC2, stage 2 conf vol vCu, unblocked vol 344 136 151 tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B A	vC, conflicting volume	344	136			151			
vCu, unblocked vol 344 136 151 tC, single (s) 6.4 6.2 4.1 tC, 2 stage (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B A	vC1, stage 1 conf vol								
tC, single (s) tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Approach LOS B A	vC2, stage 2 conf vol								
tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach LOS B	vCu, unblocked vol	344	136			151			
tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach LOS B	tC, single (s)	6.4	6.2			4.1			
p0 queue free % 86 94 99 cM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B A	tC, 2 stage (s)								
CM capacity (veh/h) 641 911 1431 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B A	tF (s)	3.5	3.3			2.2			
Direction, Lane # WB 1 NB 1 SB 1 Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B A	p0 queue free %	86	94			99			
Volume Total 150 141 200 Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B 0.0 0.4	cM capacity (veh/h)	641	911			1431			
Volume Left 92 0 9 Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B 0.0 0.4	Direction, Lane #	WB 1	NB 1	SB 1					
Volume Right 58 31 0 cSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B A	Volume Total	150	141	200					
CSH 724 1700 1431 Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B	Volume Left	92	0	9					
Volume to Capacity 0.21 0.08 0.01 Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B	Volume Right	58	31	0					
Queue Length 95th (m) 6.2 0.0 0.2 Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B	cSH	724	1700	1431					
Control Delay (s) 11.3 0.0 0.4 Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B	Volume to Capacity	0.21	0.08	0.01					
Lane LOS B A Approach Delay (s) 11.3 0.0 0.4 Approach LOS B	Queue Length 95th (m)	6.2	0.0	0.2					
Approach Delay (s) 11.3 0.0 0.4 Approach LOS B	Control Delay (s)	11.3	0.0	0.4					
Approach Delay (s) 11.3 0.0 0.4 Approach LOS B	Lane LOS	В		Α					
	Approach Delay (s)	11.3	0.0	0.4					
Intersection Summary	Approach LOS	В							
	Intersection Summary								
Average Delay 3.6				3.6					
Intersection Capacity Utilization 29.8% ICU Level of Service		zation			IC	U Level o	f Service		
Analysis Period (min) 15							2 2. 1.03		

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			4
Traffic Volume (veh/h)	56	16	80	18	5	122
Future Volume (Veh/h)	56	16	80	18	5	122
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	63	18	90	20	6	137
Pedestrians	10					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	1					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	259	110			120	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	259	110			120	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	91	98			100	
cM capacity (veh/h)	719	941			1468	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	81	110	143			
Volume Left	63	0	6			
Volume Right	18	20	0			
cSH	759	1700	1468			
Volume to Capacity	0.11	0.06	0.00			
Queue Length 95th (m)	2.9	0.00	0.00			
Control Delay (s)	10.3	0.0	0.1			
Lane LOS	В	0.0	0.5 A			
Approach Delay (s)	10.3	0.0	0.3			
Approach LOS	В	0.0	0.0			
	D					
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utiliz	ation		21.2%	IC	U Level o	f Service
Analysis Period (min)			15			

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Intersection				
Intersection Delay, s/veh	27.0			
Intersection LOS	D			
Approach	EB	WB	NB	SB
Entry Lanes	1	1	1	1
Conflicting Circle Lanes	1	1	1	1
Adj Approach Flow, veh/h	438	253	701	221
Demand Flow Rate, veh/h	446	260	713	225
Vehicles Circulating, veh/h	231	523	427	273
Vehicles Exiting, veh/h	267	617	250	510
Follow-Up Headway, s	3.186	3.186	3.186	3.186
Ped Vol Crossing Leg, #/h	0	0	0	0
Ped Cap Adj	1.000	1.000	1.000	1.000
Approach Delay, s/veh	10.6	10.9	49.5	7.1
Approach LOS	В	В	E	А
Lane	Left	Left	Left	Left
Designated Moves	LTR	LTR	LTR	LTR
Assumed Moves	LTR	LTR	LTR	LTR
RT Channelized				
Lane Util	1.000	1.000	1.000	1.000
Critical Headway, s	5.193	5.193	5.193	5.193
Entry Flow, veh/h	446	260	713	225
Cap Entry Lane, veh/h	897	670	737	860
Entry HV Adj Factor	0.982	0.975	0.983	0.984
Flow Entry, veh/h	438	253	701	221
Cap Entry, veh/h	880	653	725	846
V/C Ratio	0.497	0.388	0.967	0.262
Control Delay, s/veh	10.6	10.9	49.5	7.1
LOS	В	В	E	A
95th %tile Queue, veh	3	2	15	1

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	†	1		¥	
Traffic Volume (veh/h)	89	490	408	71	76	39
Future Volume (Veh/h)	89	490	408	71	76	39
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	96	527	439	76	82	42
Pedestrians					4	<u> </u>
Lane Width (m)					3.5	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	519				1200	481
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	519				1200	481
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	91				56	93
cM capacity (veh/h)	1054				187	587
Direction, Lane #	EB 1	EB 2	WB 1	SB 1		
Volume Total	96	527	515	124		
Volume Left	96	0	0	82		
Volume Right	0	0	76	42		
cSH	1054	1700	1700	243		
Volume to Capacity	0.09	0.31	0.30	0.51		
Queue Length 95th (m)	2.4	0.0	0.0	21.2		
Control Delay (s)	8.8	0.0	0.0	34.3		
Lane LOS	Α	0.0	0.0	D D		
Approach Delay (s)	1.3		0.0	34.3		
Approach LOS	1.0		0.0	D D		
••						
Intersection Summary						
Average Delay			4.0			
Intersection Capacity Utiliz	ation		47.4%	IC	U Level c	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	W		f.			र्स	
Traffic Volume (veh/h)	45	22	422	57	15	172	
Future Volume (Veh/h)	45	22	422	57	15	172	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Hourly flow rate (vph)	48	24	454	61	16	185	
Pedestrians			1				
Lane Width (m)			3.5				
Walking Speed (m/s)			1.2				
Percent Blockage			0				
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	702	484			515		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	702	484			515		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	88	96			98		
cM capacity (veh/h)	396	587			1061		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	72	515	201				
Volume Left	48	0	16				
Volume Right	24	61	0				
cSH	444	1700	1061				
Volume to Capacity	0.16	0.30	0.02				
Queue Length 95th (m)	4.6	0.0	0.4				
Control Delay (s)	14.7	0.0	0.8				
Lane LOS	В		Α				
Approach Delay (s)	14.7	0.0	0.8				
Approach LOS	В						
Intersection Summary							
Average Delay			1.5				
Intersection Capacity Utiliz	zation		36.2%	IC	U Level o	of Service	
Analysis Period (min)			15				

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	47	23	34	18	21	16	44	142	22	17	96	42
Future Volume (vph)	47	23	34	18	21	16	44	142	22	17	96	42
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	52	26	38	20	23	18	49	158	24	19	107	47
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	116	61	231	173								
Volume Left (vph)	52	20	49	19								
Volume Right (vph)	38	18	24	47								
Hadj (s)	-0.11	-0.11	0.00	-0.14								
Departure Headway (s)	4.8	4.9	4.6	4.5								
Degree Utilization, x	0.16	0.08	0.29	0.22								
Capacity (veh/h)	683	662	756	758								
Control Delay (s)	8.7	8.3	9.4	8.7								
Approach Delay (s)	8.7	8.3	9.4	8.7								
Approach LOS	Α	Α	Α	Α								
Intersection Summary												
Delay			9.0									
Level of Service			Α									
Intersection Capacity Utilizat	tion		35.2%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	W	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	64	24	46	64	48	62
Future Volume (vph)	64	24	46	64	48	62
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86
Hourly flow rate (vph)	74	28	53	74	56	72
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total (vph)	102	127	128			
Volume Left (vph)	0	53	56			
Volume Right (vph)	28	0	72			
Hadj (s)	-0.16	0.08	-0.25			
Departure Headway (s)	4.2	4.4	4.2			
Degree Utilization, x	0.12	0.15	0.15			
Capacity (veh/h)	833	786	819			
Control Delay (s)	7.7	8.2	7.9			
Approach Delay (s)	7.7	8.2	7.9			
Approach LOS	Α	Α	Α			
Intersection Summary						
Delay			7.9			
Level of Service			А			
Intersection Capacity Utiliz	zation		26.8%	IC	U Level c	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ.			4
Traffic Volume (veh/h)	52	5	138	62	6	107
Future Volume (Veh/h)	52	5	138	62	6	107
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	55	5	147	66	6	114
Pedestrians	19					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	2					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	325	199			232	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	325	199			232	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	92	99			100	
cM capacity (veh/h)	660	834			1327	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	60	213	120			
Volume Left	55	0	6			
Volume Right	5	66	0			
cSH	671	1700	1327			
Volume to Capacity	0.09	0.13	0.00			
Queue Length 95th (m)	2.3	0.0	0.1			
Control Delay (s)	10.9	0.0	0.4			
Lane LOS	В	0.0	Α			
Approach Delay (s)	10.9	0.0	0.4			
Approach LOS	В	0.0	J. 1			
Intersection Summary						
			1.0			
Average Delay	zotion		1.8	10	all and -	of Condos
Intersection Capacity Utiliz	<u>'</u> all011		22.4%	IC	CU Level o	or Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	15	20	26	18	40	5	36	73	29	9	63	28
Future Volume (Veh/h)	15	20	26	18	40	5	36	73	29	9	63	28
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	16	21	28	19	43	5	38	78	31	10	67	30
Pedestrians		10			19							
Lane Width (m)		3.5			3.5							
Walking Speed (m/s)		1.2			1.2							
Percent Blockage		1			2							
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	308	316	92	329	316	112	107			128		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	308	316	92	329	316	112	107			128		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	97	96	97	97	92	99	97			99		
cM capacity (veh/h)	578	570	963	559	570	931	1484			1448		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	65	67	147	107								
Volume Left	16	19	38	10								
Volume Right	28	5	31	30								
cSH	694	584	1484	1448								
Volume to Capacity	0.09	0.11	0.03	0.01								
Queue Length 95th (m)	2.5	3.1	0.6	0.2								
Control Delay (s)	10.7	12.0	2.1	0.8								
Lane LOS	В	В	Α	Α								
Approach Delay (s)	10.7	12.0	2.1	0.8								
Approach LOS	В	В										
Intersection Summary												
Average Delay			4.9									
Intersection Capacity Utilizati	on		26.1%	IC	CU Level	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		î,			4
Traffic Volume (veh/h)	52	16	354	90	27	135
Future Volume (Veh/h)	52	16	354	90	27	135
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	56	17	381	97	29	145
Pedestrians	10					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	1					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	642	440			488	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	642	440			488	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	87	97			97	
cM capacity (veh/h)	423	617			1077	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	73	478	174			
Volume Left	56	0	29			
Volume Right	17	97	0			
cSH	456	1700	1077			
Volume to Capacity	0.16	0.28	0.03			
Queue Length 95th (m)	4.5	0.20	0.03			
Control Delay (s)	14.4	0.0	1.6			
Lane LOS	В	0.0	A			
Approach Delay (s)	14.4	0.0	1.6			
Approach LOS	В	0.0	1.0			
•	D					
Intersection Summary						
Average Delay			1.8			
Intersection Capacity Utiliza	ation		40.7%	IC	U Level of	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			र्स
Traffic Volume (veh/h)	35	11	294	60	18	127
Future Volume (Veh/h)	35	11	294	60	18	127
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	38	12	316	65	19	137
Pedestrians	10					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	1					
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	534	358			391	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	534	358			391	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	92	98			98	
cM capacity (veh/h)	495	685			1169	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	50	381	156			
Volume Left	38	0	19			
Volume Right	12	65	0			
cSH	530	1700	1169			
Volume to Capacity	0.09	0.22	0.02			
Queue Length 95th (m)	2.5	0.0	0.4			
Control Delay (s)	12.5	0.0	1.1			
Lane LOS	В	3.0	A			
Approach Delay (s)	12.5	0.0	1.1			
Approach LOS	В	0.0				
Intersection Summary						
			1 /			
Average Delay	rotion		1.4	10	المديمالا	of Comiles
Intersection Capacity Utiliz	2au0N		31.9%	IC	U Level (of Service
Analysis Period (min)			15			

Appendix G
Signal Warrant Analysis

Signal Warrant Calculation (OTM Book 12 - Justification 7)

Maior Ctro-t-				D1	Ctroot M			1/0:	VOLUME AM PM FACTOR						
Major Street:				Brock	Street W			1A -		AM 1,096	PM 1,173	n/a	568		
Minor Street:				Cen	tre Road				Minor	215	1,173	25%	83		
								2A -	Major	881	1,058	25%	485		
Comment			Future	Total (20	28) Traffic Co	ndition		2B -	Crossi	100	121	25%	55		
Number of Approach	es:				1		2 X			s factor rela					
Tee Intersection Conf	figuratio	n:			Yes	1 X	No 🗌			nt hours" to peak hours		ge of the "a	m and		
Flow Condition:	-				Fre	<u> </u>									
					Restricted F	low (Urba	n) X								
OVERALL WARRANT	i	COMB	120% S 100% S O 80% S	Satisfied: Satisfied: Satisfied:	Yes Yes Yes	'	No X V	Narrant for r Narrant for e Narrant for e	existing existing	intersection	on with fo on with ex	recast tra	fic *		
			80% 8	Satisfied:	Yes	∐ '	No X	Consider full	undergr	ound provis	ions if 100	% for forec	ast traffi		
WARRANT 1 - MINIMU APPROACH LANES	-	ICULAR 1 REST.	2 OR	MORE	AVERAGE		% Satis		es es	No X					
FLOW CONDITION	FLOW	FLOW	FLOW	FLOW X	HOUR PERIOD	100	% Satis % Satis % Satis	fied: Y	es es	No X No X					
ALL APPROACHES	480	720 % FUL	600 FILLED	900	568 63%										
APPROACH LANES		1		MORE	AVERAGE										
FLOW CONDITION		REST. FLOW			HOUR PERIOD										
MINOR STREET	180	255	180	255	83]									
APPROACHES		% FUL	FILLED		33%	J									
WARRANT 2 - DELAY	TO CR	OSS TR	AFFIC												
APPROACH LANES		1	2 OR	MORE	AVERAGE	150	% Satis	fied: Y	es	No X					
FLOW CONDITION	1	REST. FLOW		REST. FLOW X	HOUR PERIOD	100	% Satis % Satis % Satis	fied: Y	es es	No X No X No X					
MAJOR STREET	480	720	600	900	485	1			لسا	<u></u>					
APPROACHES			FILLED		54%										
APPROACH LANES		1		MORE	AVERAGE										
FLOW CONDITION		REST. FLOW			HOUR PERIOD										
TRAFFIC CROSSING	50	75	120	170	55]									
MAJOR STREET		% FUI	FILLED		32%	1									

¹A - MINIMUM VEHICULAR VOLUME: Total vehicle volume on all approaches for average day

¹B - MINIMUM VEHICULAR VOLUME: Total vehicle volume on minor streets

²A - DELAY TO CROSS TRAFFIC: Total vehicle volume on major street for average day

²B - DELAY TO CROSS TRAFFIC: Total vehicle and pedestrian volume crossing major street; comprising: (1) lefts from both minor streets, (2) heaviest through from minor street, (3) 50% of heavier left turn from major street when following criteria met: (a) left turn volume >120 and (b) left turn volume plus opposing volume > 720, (4) pedestrians crossing the major street.